Traffic Impact Study and Transportation Management Plan West Rosslyn Development

Arlington County, VA

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EXECUTIVE SUMMARY

This report presents findings of a transportation impact study conducted for the proposed development in West Rosslyn in Arlington, Virginia. This study was developed in accordance with VDOT and Arlington County transportation impact analysis guidelines.

Site Location and Study Area

The site is located in the West Rosslyn area of Arlington, Virginia, south of 18th Street N, north of Wilson Boulevard, east of N Quinn Street, and west of N Oak Street. Regional access to the site is provided via I-66/US29 and the George Washington Memorial Parkway from the north and east, US 50 (Arlington Blvd) to the south and Wilson Boulevard/Clarendon Boulevard to the west. Immediate vehicular access to the site will be provided via a new north-south street which will bisect the site and connect Wilson Boulevard to 18th Street N.

Description of Proposed Development

According to Arlington County's General Land Use Plan (GLUP), this site is currently listed as "High Office-Apartment-Hotel" and "Public Space." The new development will be divided in two parts, the eastern parcel will include one residential tower and space for ground-floor retail. The eastern parcel will also include the redevelopment of the existing fire station. The western parcel will include a tower with office space and ground floor retail, and the redevelopment of the existing Rosslyn Highlands Park. The proposed project build-out year is 2019.

Principal Findings, Conclusions, and Recommendations

The analysis presented in this report supports the following major conclusions:

Existing Conditions (2016)

- The subject site is well-served by transit:
 - The site is approximately less than one-half mile walking distance from the Rosslyn Metro station, which serve the Blue, Orange, and Silver lines.
 - There are four bus stops within one block of the site. These stops are directly served by WMATA (Metrobus)
 and Arlington Transit (ART) routes.
- Vehicular traffic operations in the study area are good overall. All intersection movements within the study area operate at the target Level of Service (LOS) D or better during the AM, PM, and Saturday peak hours, with the exception of the following:
 - N Quinn Street and Wilson Boulevard
 - Southbound right lane (AM peak hour)
 - N Oak Street and Key Boulevard
 - Northbound left/right lane (AM peak hour)
 - N Oak Street and Wilson Boulevard
 - Northbound left/thru lane (AM peak hour)
 - N Nash Street (east) and Key Boulevard (AM peak hour)

- Eastbound left lane (AM and PM peak hour)
- N Nash Street and Lee Highway
 - Southbound left lane (AM peak hour)
- None of the movements listed above cause the intersection to operate below acceptable LOS thresholds with the exception of the intersection of N Nash Street (east) and Key Boulevard in the AM peak hour. Signalization as mitigation at this intersection is considered with a signal warrant analysis.

Future Conditions without Development (2019)

- Historical roadway volumes indicate negative growth has occurred in the study area; however, a conservative 0.5% annual background growth was applied at the study intersections as per agreement with VDOT and Arlington County. Ten planned background developments in the vicinity of the site were also taken into consideration. The trips generated by these sites were added to the roadway network to reflect future conditions without the proposed development in 2019.
- Under future without development conditions, all movements at the study intersections operate at acceptable levels of service consistent with the Existing Conditions scenario, with the exception of the following:
 - N Quinn Street and Key Boulevard
 - Southbound left/thru lane (AM peak hour)
 - N Pierce Street and Clarendon Boulevard
 - Southbound left/thru lane (AM peak hour)
 - N Oak Street and Clarendon Boulevard
 - Eastbound left lane (AM peak hour)
 - N Nash Street (east) and Key Boulevard (PM peak hour)
 - N Nash Street and Lee Highway
 - Southbound left lane (PM peak hour)
- None of the movements listed above cause the intersection to operate below acceptable LOS thresholds with the exception of the intersection of N Nash Street (east) and Key Boulevard in the PM peak hour. Signalization as mitigation at this intersection is considered with a signal warrant analysis.

Future Conditions with Development (2019)

- The proposed mixed-use development will generate approximately 268 net trips in the AM peak hour, 348 net trips in the PM peak hour, and 287 net trips in the Saturday peak hour.
- Under future with development conditions, all intersection movements within the study area operate at acceptable levels of service consistent with the Future without Development scenario, with the exception of the following:
 - N Quinn Street and Key Boulevard (AM peak hour)
 - Southbound left/thru lanes (AM peak hour)
 - N Pierce Street and Clarendon Boulevard

- Southbound left/thru lane (PM peak hour)
- N Oak Street and Wilson Boulevard (AM peak hour)
 - Northbound left/thru lane (PM and Saturday peak hour)
- N Oak Street and Clarendon Boulevard
 - Eastbound left lane (PM peak hour)
- All movements caused to operate below acceptable LOS thresholds by the proposed development have been mitigated with signal timing adjustments. No additional mitigations are required.

Future Conditions with Development (2025)

- There are no additional site-added trips between 2019 and 2025. A regional growth rate of 0.5% annually has been applied between the 2019 and 2025 scenarios
- Under future (2025) with development conditions, all intersection movements within the study area continue to operate consistent with future (2019) with development results with the exception of the following:
 - N Quinn Street and Key Boulevard
 - Westbound left/thru/right lane (AM peak hour)
 - Southbound left/thru lane (AM peak hour)
- Other movements caused to operate below acceptable LOS thresholds by the proposed development have been mitigated with signal timing adjustments. Although movements at the intersection of N Quinn Street and Key Boulevard exceeds County standards, the 2025 analysis was done as a planning scenario; therefore, no mitigation is proposed at this intersection.

Transportation Management Plan

• A TMP will be required for the project based on the County's requirements. The initial framework for a TMP is included in this report. The ultimate TMP will be determined per the approved site plan conditions.

INTRODUCTION AND SUMMARY

This report presents the findings of a traffic impact study conducted for the proposed West Rosslyn mixed use development in Arlington, VA. The site is currently occupied by a park, fire station, and office building with a parking garage. The new development will be developed in two parts. The east side will include one residential tower, and space for ground-floor retail. The eastern development will also include the redevelopment of the existing Fire Station #10. The western parcel will include a tower with approximately 400,000 square feet of office, additional ground floor retail space, and redevelopment of existing Rosslyn Highlands Park. Access to the development will be provided from a newly created street bisecting the site connecting Wilson Boulevard to 18th Street N. The east parcel will have loading areas off of 18th Street N or an alley from 18th Street N that is shared with the fire station. The fire station will also have access to Wilson Boulevard for a pull through configuration from 18th Street. The western development parcel will have loading access provided off of the new street connection. The proposed project build-out year is 2019.

Study Objectives

The objectives of this study are to evaluate the transportation network in the vicinity of the site and identify any potential transportation impacts that may result from the proposed redevelopment. Elements of this report include a description of the proposed development, an evaluation of the existing transportation network, and evaluations of the future transportation network with and without the proposed development.

Study Tasks

The following tasks were completed as part of this study.

- A scoping meeting was conducted on March 15, 2016, and a scope dated March 31, 2016 was agreed upon by Gorove/Slade, VDOT, and representatives from Arlington County. This scope includes discussions about the parameters of the study and relevant background information. A copy of the scoping document confirming the agreed upon parameters and assumptions is included in Appendix A.
- Field reconnaissance in the vicinity of the site was performed to collect information related to the existing traffic controls, roadway geometry, and traffic flow characteristics.
- Traffic counts at the study area intersections were conducted on Wednesday, April 1, 2015, Tuesday, February 23, 2016, Thursday, March 10th, 2016, Tuesday, March 29th, 2016, and Thursday, April 14th, 2016 during the weekday morning and evening peak periods. Traffic counts at the study area intersections were conducted on Saturday, March 5, 2016 and Saturday, April 2nd, 2016 during the Saturday peak period. All counts were collected during typical conditions.
- As outlined in the scoping document, 10 planned developments in the vicinity of the proposed project were also assumed to be in place for the future conditions analysis. As determined based on historical traffic patterns and projected traffic volumes, a growth rate of 0.5 percent per year was applied to account for a regional increase in background traffic.
- Proposed site traffic volumes were generated based on the methodology outlined in <u>Trip Generation</u>, 9th <u>Edition</u>
 published by the Institute of Transportation Engineers (ITE).
- Intersection capacity analyses were performed using the software package Synchro, Version 9.1 based on the <u>Highway Capacity Manual</u> (HCM) 2000 and 2010 methodology. Traffic analyses were performed for the existing

conditions (2016), future (2019) with and without development conditions, and future (2025) with development conditions.

 An initial Transportation Management Plan framework was developed as a TMP will be necessary to meet County requirements.

Data Sources

Sources of data for this study include Arlington County, the Virginia Department of Transportation (VDOT), the Institute of Transportation Engineers (ITE) <u>Trip Generation</u>, <u>9th Edition</u>, Penzance, Hickok Cole Architects, and the office files and field reconnaissance efforts of Gorove/Slade Associates, Inc.

PROPOSED DEVELOPMENT AND CONTEXT

Site Description

Site Location

The site is located in the Rosslyn area of Arlington, Virginia between 18th Street N, Wilson Boulevard, N Quinn Street, and N Oak Street. Vehicular access to the development will be provided via a newly created street bisecting the east and west parcels and connecting Wilson Boulevard to 18th Street N. The site location is shown in Figure 1.

The site location falls within the bounds of the Western Rosslyn Area Planning Study (WRAPS), the Rosslyn Coordinated Redevelopment District (RCRD), and the Rosslyn Metro Station Area (RMSA). WRAPS was a study undertaken in 2015 by the Arlington County Board to consider the various uses of the redevelopment of the area surrounding the Rosslyn Highlands Park, school, and firehouse. Due to the proximity of the development's location in the WRAPS area, coordination between the West Rosslyn development and WRAPS was undertaken to make sure the Board's goals were met. The County Board set out goals for reinvigorating the area in a public-private partnership designed to facilitate the rebuilding of the firehouse and public park as well as making the West Rosslyn area a destination day and night for students, residents, and office workers of all income levels. The proposed West Rosslyn development will meet the Board's goal of creating privately-developed office, residential, and retail space.

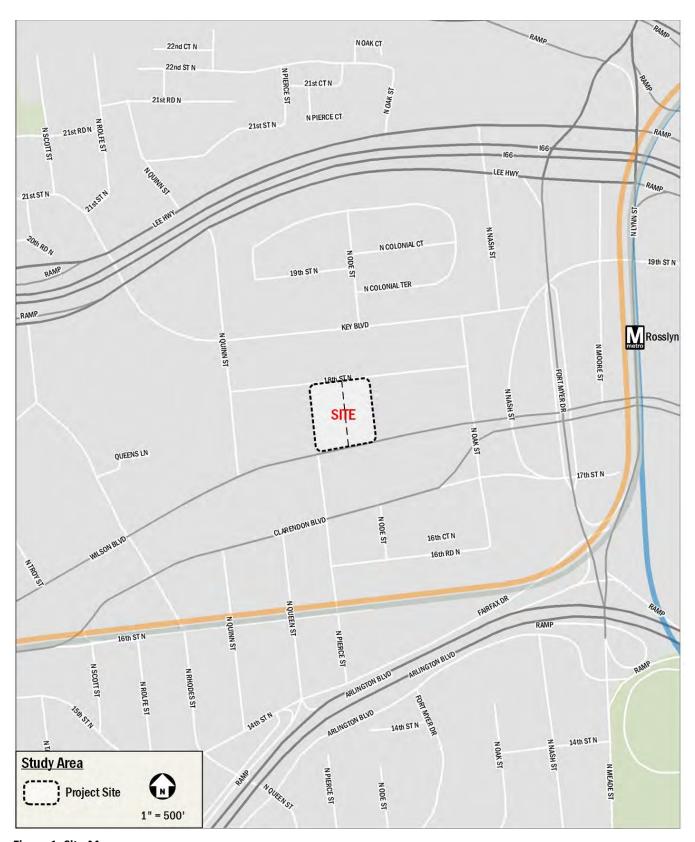


Figure 1: Site Map

Parcel Information

The existing parcels are owned by the Penzance Company and the County Board of Arlington. The site consists of the Rosslyn Highlands Park and Fire Station #10 in the western parcels (owned by the County) and contains approximately 54,000 sf of gross floor area. 1555 Wilson Boulevard in the eastern parcels and contains approximately 156,825 sf of gross floor area. A parcel map showing the location of the property is presented in Figure 2. The various parcels owned by Penzance and the County Board add up to a total of approximately 2.7 acres.



Figure 2: Parcel Map (Source: Arlington County Real Estate Map, September 2015)

General Land Use Plan Recommendations

According to Arlington County's General Land Use Plan (GLUP), this site is currently listed as "High Office-Apartment-Hotel" and "Public Space." The GLUP map for the site is shown in Figure 3. The West Rosslyn Area Planning Study (WRAPS) recommends in the short-term to change the portion of public space where the western parcel of development is to be built to "High Office-Apartment-Hotel." The Arlington Zoning Map shows the site falling within the C-2 (Service Commercial-Community Business Districts), C-3 (Office), RA6-15 (Apartment Dwelling Districts), and S-3A (Special Districts) zones, as seen in Figure 4.

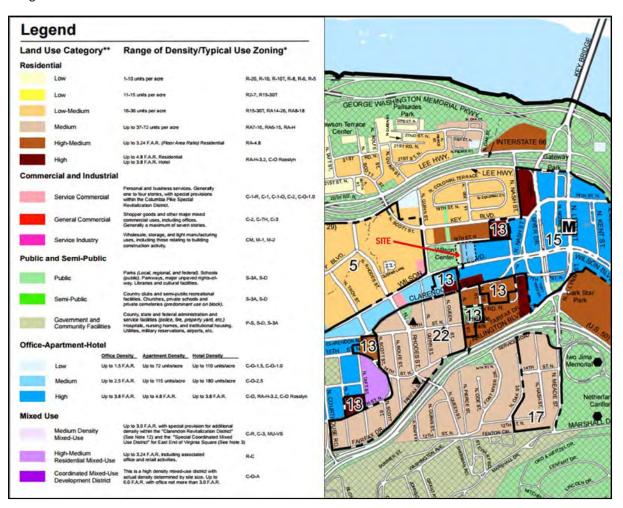


Figure 3: Planned Land Uses (Source: Arlington General Land Use Plan (GLUP), July 2015)

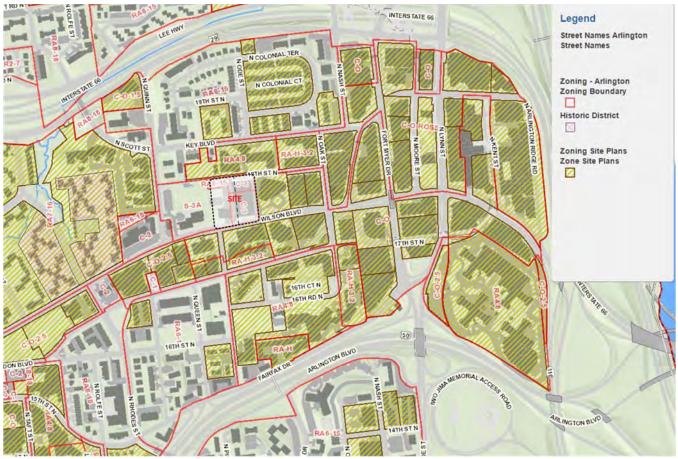


Figure 4: Zoning Map (Source: Arlington County)

Proposed Site Plan

The proposed West Rosslyn development will be divided in two parts, with the eastern parcel will include one residential tower and space for ground-floor retail. The eastern parcel will also include the redevelopment of the existing fire station. The western parcel will include a tower with office space and ground floor retail, and the redevelopment of the existing Rosslyn Highlands Park. The proposed site plan is presented in Figure 5. Vehicular access to the site will be provided via a new north-south street connecting Wilson Boulevard with 18th Street N.



Figure 5: Site Plan

Scope and Limits of the Study Area

The study area is bound by 18th Street N, Wilson Boulevard, N Quinn Street, and N Oak Street. Regional access to the site is provided via I-66/US-29 and George Washington Memorial Parkway from the north and east, US 50 (Arlington Blvd) to the south and Wilson Boulevard/Clarendon Boulevard to the west. The following intersections were identified for inclusion in this study, as shown in Figure 6.

- 1. N Quinn Street and Key Boulevard and N Scott Street
- 2. N Quinn Street and 18th Street N
- 3. N Quinn Street and Wilson Boulevard
- 4. N Pierce Street and Wilson Boulevard
- 5. N Pierce Street and Clarendon Boulevard
- 6. Wilson Boulevard and New Street
- 7. 18th Street N and New Street
- 8. N Oak Street and Key Boulevard
- 9. N Oak Street and 18th Street N
- 10. N Oak Street and Wilson Boulevard
- 11. N Oak Street and Clarendon Boulevard
- 12. N Nash Street and Key Boulevard
- 13. N Nash Street/19th Street N and Key Boulevard
- 14. N Nash Street and Key Highway
- 15. N Fort Myer Drive and Lee Highway

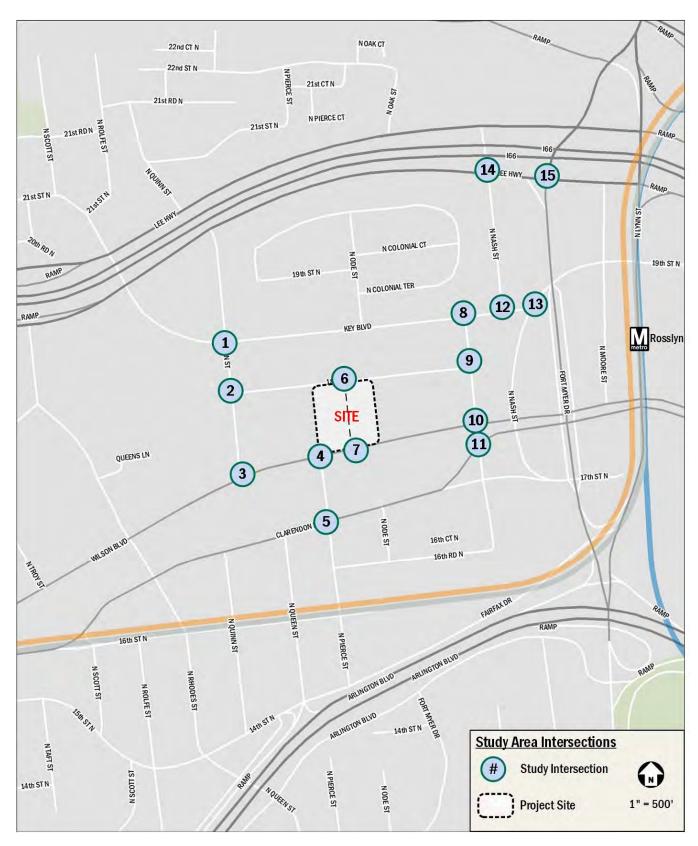


Figure 6: Study Area

EXISTING CONDITIONS (2016)

Existing Transit Service

The study area is served by numerous transit options under existing conditions as shown in Figure 7 and described in the following sections.

Metrorail

There are two Metro stations within one-half mile walking distance of the site:

Rosslyn Metro Station— The Rosslyn Metro station is located between 19th Street N and Wilson Blvd on N Moore Street, with the closest entrance on N Fort Myer Drive between 19th Street N and Wilson Blvd. The station is located within a ¼ mile walking distance from the site via Wilson Boulevard to N Fort Myer Drive to the station entrance on N Fort Myer Drive. The Rosslyn Metro serves the Blue, Orange, and Silver lines and is a major transfer point for locations to the south (Arlington County, City of Alexandria), west (Fairfax County), and north (Washington, D.C.). Daily ridership at this station in 2015 was approximately 14,800 boardings on weekdays according to the publication Metrorail Average Weekday Passenger Boardings (WMATA, December 2015). Based on the Metro Station Access & Capacity Analysis report dated April 2008, neither the Rosslyn nor Court House Metro Station are currently or predicted to have capacity issues.

The Rosslyn Metro station is served by 16 bus routes including routes provided by WMATA (Metrobus), Arlington County Transit (ART), Loudoun County Transit, DC Circulator, and Georgetown University. The majority of the routes are bidirectional and have service throughout the day; however, some of these routes are rush hour only. A list of the routes, frequencies, and destinations are provided in Figure 7 and Table 1.

Court House Metro Station - The Court House Metro station is located southwest of the development site at the intersection of Clarendon Boulevard and 15th Street N on N Uhle Street. It can be reached by walking south from the site on Wilson Boulevard to N Uhle Street. There are sidewalks along this route. The Court House Metro serves Orange and Silver lines, and average daily ridership at the station in 2015 was approximately 7,448 boardings on weekdays according to the publication Metrorail Average Weekday Passenger Boardings (WMATA, December 2015).

The Court House Metro station is the further of the two Metro stations. It is expected that many of the tenants, customers, and employees of the propose development will utilize the closer Rosslyn Metro Station. The station is served by ten bus routes including four routes provided by WMATA (Metrobus) and six routes provided by ART.

Based on the *Metro Station Access & Capacity Analysis* report dated April 2008, neither the Rosslyn nor Court House Metro Station are currently or predicted to have capacity issues.

Bus

There are four bus stops within one block of the site, represented in Figure 7. These stops are served by the following routes:

• WMATA (Metrobus) – Two Metrobus routes directly serve the bus stops closest to the site: 4B and 38B. These lines run westbound on Wilson Boulevard, outbound from Rosslyn Station. One block further south, the inbound lines run along Clarendon Boulevard. These routes, along with the other nearby Metrobus routes and their destinations and frequencies are presented in Table 1.

■ ART (Arlington Transit) – The 43, 45, 61A, and 61B ART bus routes directly serves the bus stops closest to the site. The 61A and 61B routes form a loop connecting the Rosslyn and Court House Metro stations and the 43 and 45 lines provide connection to Crystal City and Columbia Pike corridor, respectively. All nearby ART Routes and their frequencies are presented in Table 1.

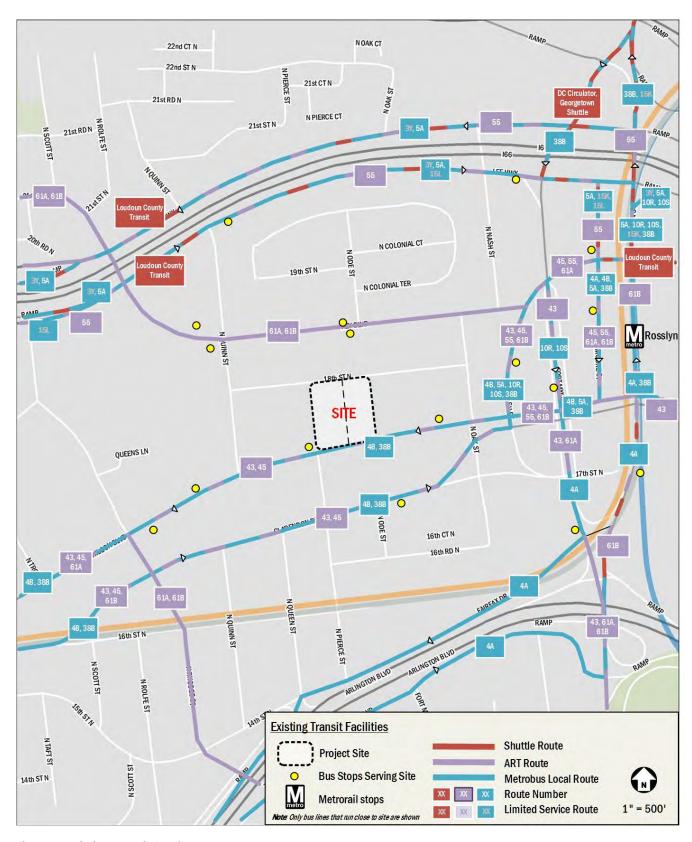


Figure 7: Existing Transit Service

Table 1: Bus Routes from Rosslyn Metro

Route Number	Route Name	Service Hours	Headway	Walking Distance to Nearest Bus Stop		
3Y (WMATA)	Lee Highway-Farragut Square Line	Weekdays: 6:52 AM – 7:32 PM	14-30 minutes	0.3 miles, 5 minutes		
4A, 4B (WMATA)	Pershing DrArlington Blvd. Line	Weekdays: 5:50 AM – 12:25 AM Weekends: 6:45 AM – 11:05 PM	4-75 minutes	<0.1 miles, 1 minute		
5A (WMATA)	D.CDulles Line	Weekdays: 5:00 AM – 12:06 AM Weekends: 5:45 AM – 12:10 AM	57-63 minutes	0.3 miles, 8 minutes		
10R, 10S (WMATA)	Hunting Point-Pentagon Line	Weekdays: 6:44 AM – 6:15 PM	28-33 minutes	0.2 miles, 4 minutes		
15K, 15L (WMATA)	Chain Bridge Road Line	Weekdays: 5:40 AM – 8:05 PM	22-40 minutes	0.3 miles, 8 minutes		
38B (WMATA)	Ballston-Farragut Square Line	Weekdays: 5:32 AM – 1:36 AM Weekends: 5:46 AM – 1:38 AM	10-40 minutes	<0.1 miles, 1 minute		
43 (ART)	Crystal City-Rosslyn-Courthouse Line	Weekdays: 6:21 AM – 7:39 PM	9-28 minutes	0.1 miles, 2 minutes		
45 (ART)	Columbia Pike-DHS/Sequoia- Rosslyn Line	Weekdays: 6:20 AM – 10:43 PM Weekends: 7:28 AM – 11:33 PM	20-43 minutes	<0.1 miles, 1 minute		
55 (ART)	East Falls Church-Lee Highway- Rosslyn Line	Weekdays: 5:26 AM – 1:15 AM Weekends: 6:13 AM – 12:35 AM	12-35 minutes	0.4 miles, 7 minutes		
61A, 61B (ART)	Rosslyn-Court House Metro Shuttle Line	Weekdays: 6:28 AM – 6:58 PM	25 minutes	0.1 miles, 2 minutes		
Loudoun County Transit	Loudoun to Washington, D.C. Line	Weekdays: 6:10 AM – 7:00 PM	2-41 minutes	0.4 miles, 7 minutes		
GUTS (Georgetown University)	Rosslyn GUTS Line	Weekdays: 5:00 AM – 12:00 AM Saturdays: 11:45 AM – 5:35 PM	10-110 minutes	0.4 miles, 7 minutes		
DC Circulator	Dupont Circle-Rosslyn Line	Sunday-Thursday: 7:00 AM – 12:00 AM Friday-Saturday 7:00 AM – 2:00 AM	10 minutes	0.4 miles, 7 minutes		

Existing Bicycle and Pedestrian Facilities

The West Rosslyn development site is situated within an urban transportation network, with quality pedestrian and bicycle access. Figure 8 shows the existing pedestrian peak hour volumes at all study area intersections. Figure 9 shows the existing conditions for bicycle travel and bicycle facilities within the study area.

There are existing sidewalks on one or both sides of the roadways within the study area. There is significant pedestrian activity within the study area as shown in Figure 8. The most heavily-used crosswalk is across Wilson Boulevard and N Oak Street, with approximately 553 pedestrians crossing at this location during the weekday AM peak hour, approximately 614 during the weekday PM peak hour, and approximately 292 in the Saturday peak hour.

The Arlington County Bike Map (Bike Arlington, July 2015) shows on-street signed routes on N Fort Myer Drive, Fairfax Drive, N Meade Street, and N Nash Street between N Fort Myer Drive and Wilson Boulevard. Bicycle lanes are provided along the majority of Wilson Boulevard (westbound) and Clarendon Boulevard (eastbound), N Quinn Street between Lee Highway and Wilson Boulevard, N Lynn Street between Lee Highway and N Fort Myer Boulevard, and Key Boulevard between Lee Highway and N Nash Street. These on-street bike routes link up to the Custis trail, which leads eastward to the Potomac Heritage and Mt. Vernon trails that connect Rosslyn to areas such as Ballston, the Memorial Bridge, the Tidal Basin, Fairfax, and Potomac Park.

Bike racks are available at the Rosslyn Metro station. There is a Capital Bikeshare station one block south of the site at the northwest corner of N Pierce Street and Clarendon Boulevard; several other Capital Bikeshare stations around the site vicinity are presented in Figure 9.

Traffic Impact Study and Transportation Management Plan – West Rosslyn

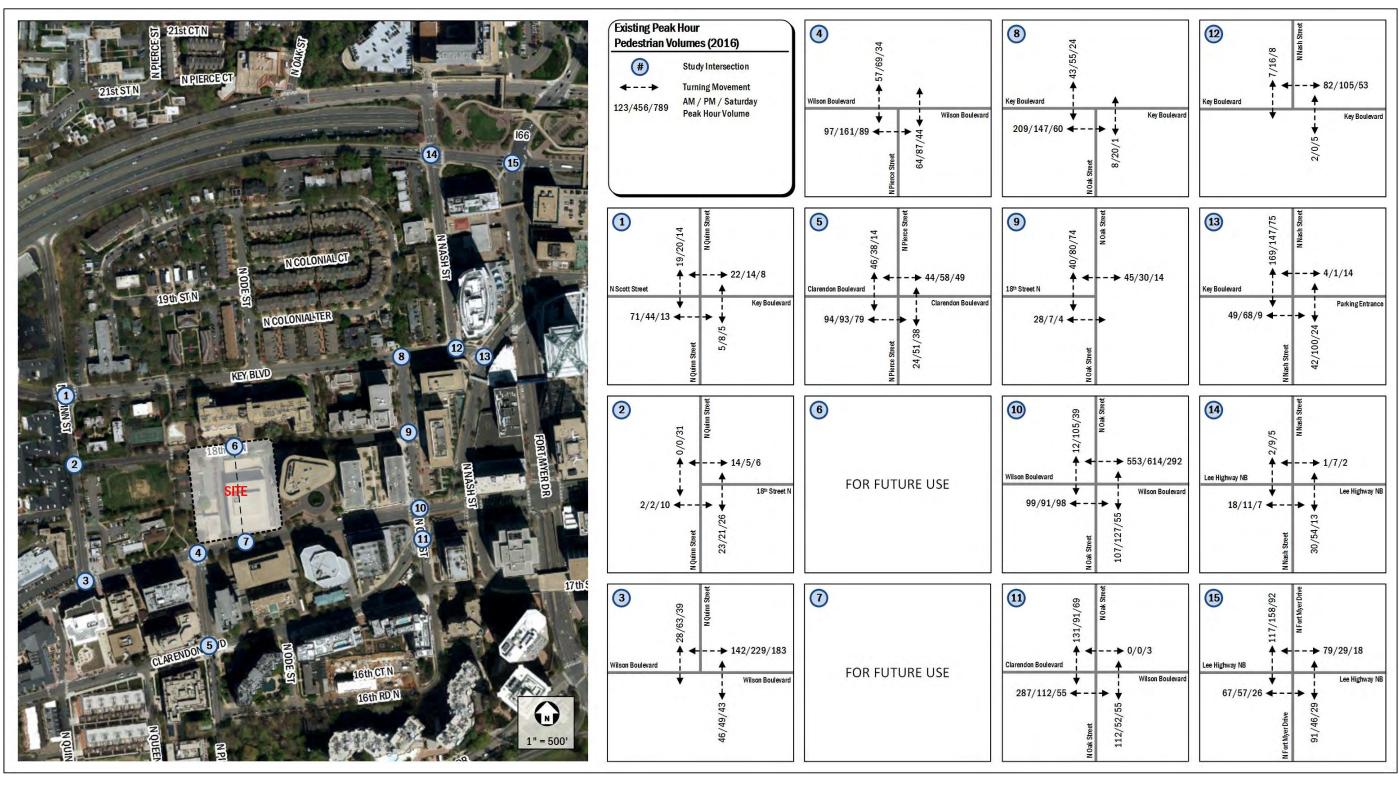


Figure 8: Existing Pedestrian Peak Hour Volumes

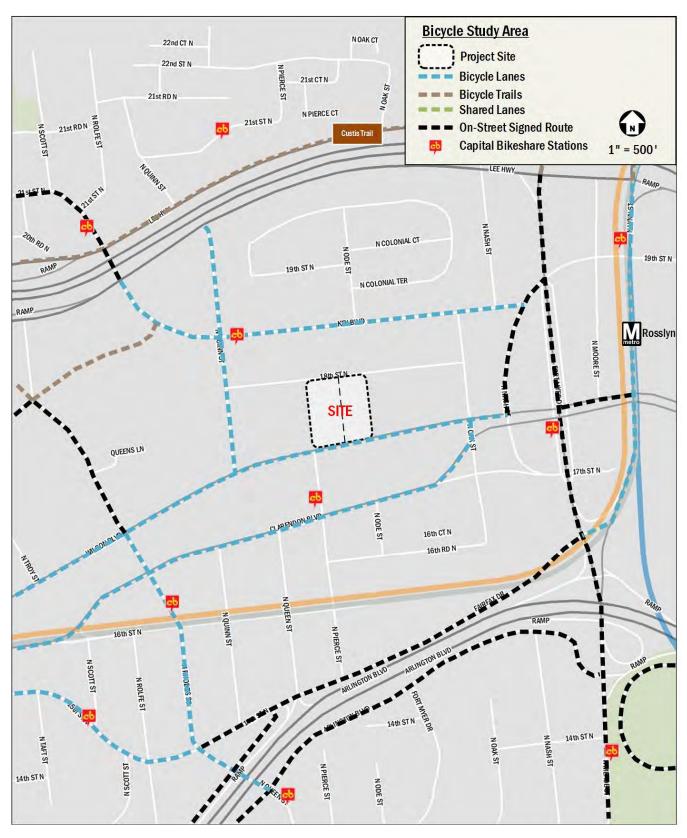


Figure 9: Nearby Bicycle Facilities (Information from Arlington County Bike Map, July 2015)

Existing Roadway Network

A description of the roadways within the study area is presented below in Table 2. The existing local roadway network including lane configurations and intersection control is detailed in and illustrated in Figure 10.

Table 2: Existing Roadway Network

Roadway	Classification*	Lanes	Speed	On-Street Parking	ADT**	
Wilson Boulevard	Other Principal Arterial (VDOT)	2-3	25 mnh	Yes	11,000	
wilson Boulevard	Other Principal Arterial (Arlington)	2-3	25 mph	res	11,000	
Clarendon Boulevard	Other Principal Arterial (VDOT)	2	25 mph	Yes	12,000	
Ciarendon Bodievard	Other Principal Arterial (Arlington)	Z	25 111011	res	12,000	
N Nash Street	Major Collector (VDOT)	2-4	Not Posted	Yes	6,800	
in nasii street	Minor Arterial (Arlington)	2-4	Not Posted	ies	0,800	
Key Boulevard	Major Collector (VDOT)	2	Not Posted	Yes	3,800	
key Boulevaru	Neighborhood Principal (Arlington)	2	Not Posted	res	3,800	
N Pierce Street	Major Collector (VDOT)	2	Not Posted	Yes	5,000	
N FIEICE Street	Neighborhood Principal (Arlington)	2	Not Fosted	163	3,000	
N Quinn Street	Major Collector (VDOT)	2	Not Posted	Yes	4,300	
N Quilli Street	Minor Arterial (Arlington)	Z	Not Posted	res	4,300	
18th Street N	Not Classified (VDOT)	2	Not Posted	Yes	1,220***	
16th Street N	Neighborhood Minor (Arlington)	Z	Not Posted	ies	1,220	
N Oak Street	Minor Arterial (VDOT)	2	Not Posted	Yes	2,510***	
N Oak Street	Neighborhood Principal (Arlington)	Z	Not Posted	res	2,510	
N Fort Myer Drive	Minor Arterial (VDOT)	2-4	30 mph	Yes	13,000	
N FOR Myer Drive	Other Principal Arterial (Arlington)	2-4	30 mpn	res	13,000	
Lea Highway (ND Lan)	Other Principal Arterial (VDOT)	2	20 h	N	16.000	
Lee Highway (NB leg)	Other Principal Arterial (Arlington)	3	30 mph	No	16,000	
N. Coott China at	Not Classified (VDOT)	2	Net Peste 1	N	4.000***	
N Scott Street	Minor Arterial (Arlington)	2	Not Posted	No	4,000***	

^{*}Information supplied from VDOT and Arlington GIS

^{**}VDOT ADT Data from 2014

^{***}ADT calculated using a k factor of 0.10

Traffic Impact Study and Transportation Management Plan – West Rosslyn

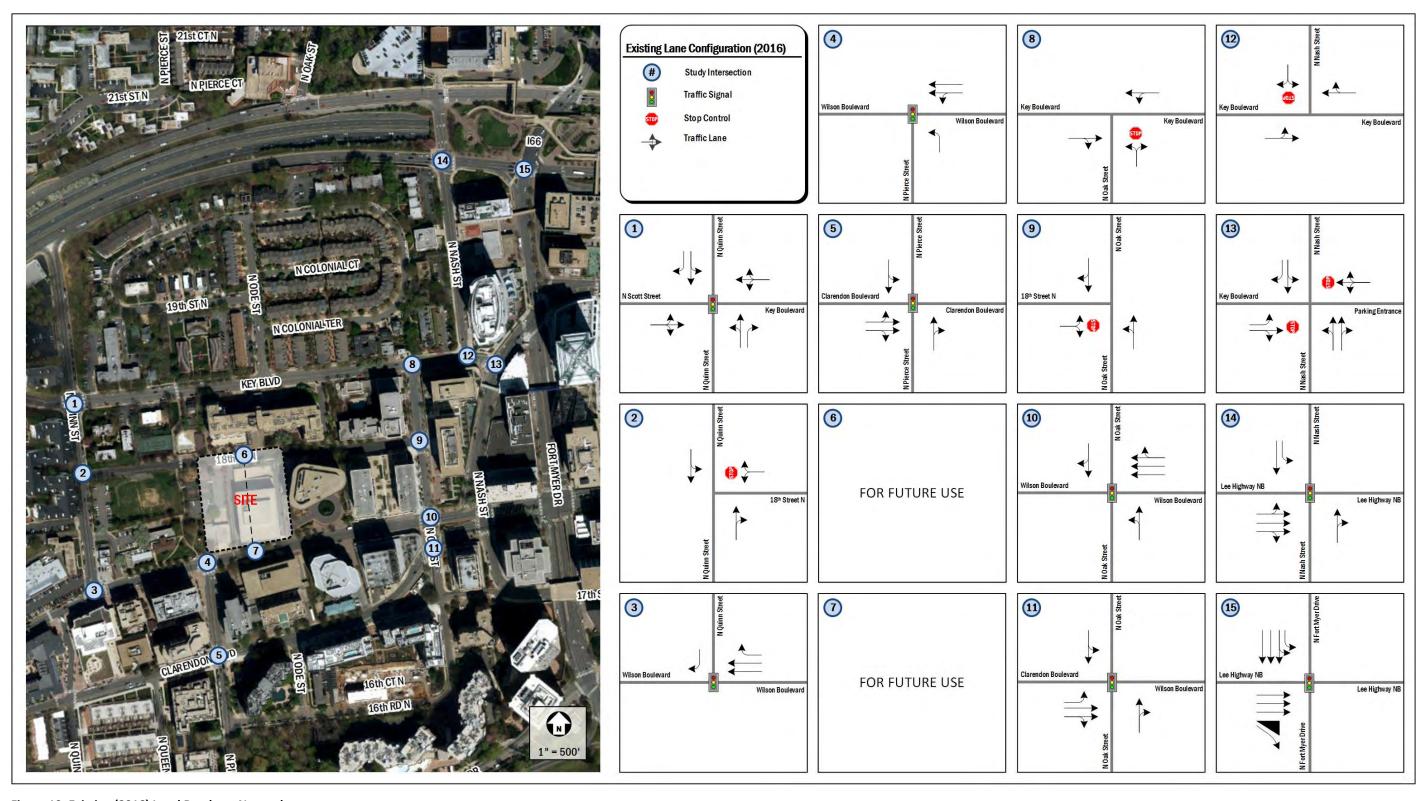


Figure 10: Existing (2016) Local Roadway Network

Existing Traffic Volumes

Weekday AM and PM peak hour traffic counts were conducted at the study area intersections on Tuesday, February 23, 2016, Thursday, April 14, 2016, and Tuesday, April 19, 2016. Counts were collected previously at the intersections of N Quinn Street & Wilson Boulevard, N Pierce Street & Wilson Boulevard, and N Oak Street & Wilson Boulevard on Wednesday, April 1, 2015. Saturday counts were taken on March 5, 2016 and Saturday, April 2, 2016. Based on these traffic counts, the weekday system peak hours occurred from 8:15 AM to 9:15 AM and 5:00 PM to 6:00 PM, with the Saturday system peak hour occurring from 1:00 PM to 2:00 PM. It should be noted that there are locations where existing volumes do not balance due to driveways located between intersections. The existing peak hour traffic volumes for the intersections within the study area are shown in Figure 11. The existing turning movement counts are included in the Appendix B.

Traffic Impact Study and Transportation Management Plan – West Rosslyn

Gorove/Slade Associates

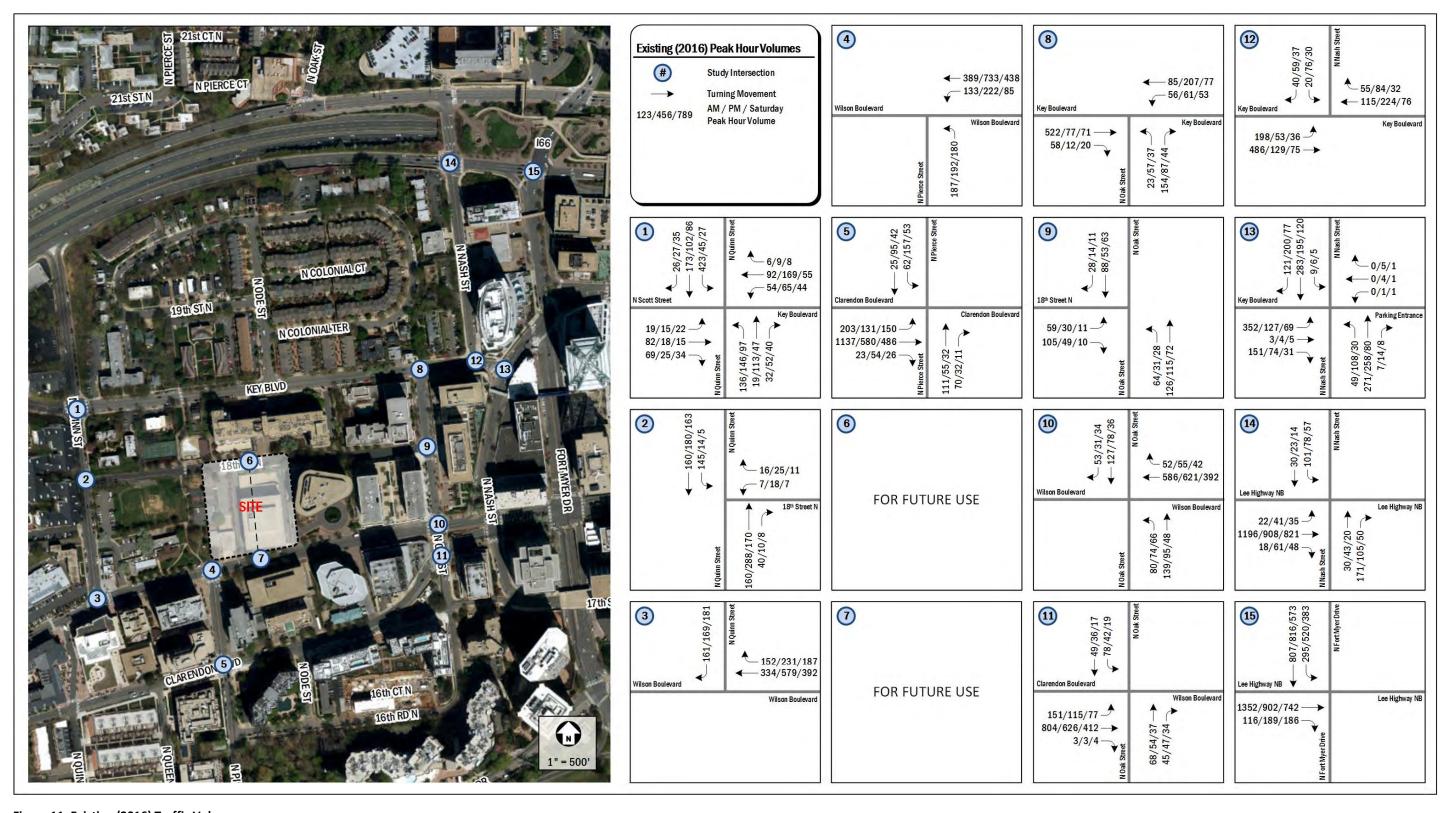


Figure 11: Existing (2016) Traffic Volumes

Existing Capacity Analysis

Capacity analyses were performed for the intersections within the study area during the weekday AM, weekday PM, and Saturday peak hours under existing conditions. *Synchro, Version 9.1* was used to analyze the study intersections based on the <u>Highway Capacity Manual</u> (HCM) methodology and include level of service, delay, and queue length comparisons for the turning movements analyzed.

Peak hour factors were applied in accordance with *Traffic Impact Analysis Regulations Administrative Guidelines* prepared by VDOT and *Traffic Operations and Safety Analysis Manual (TOSAM)* prepared by VODOT dated July 2012 and November 2015, respectively. As such, peak hour factors by approach between 0.85 and 1.00 were used for the existing year analysis. Where the calculated peak hour factor based on the existing turning movement counts was greater than 0.85, the calculated factor was applied. Where the calculated PHF was 0.85 or less, a factor of 0.85 was used.

Existing signal timings were provided by Arlington County for signalized intersections in the study area.

The results of the intersection capacity analyses are presented in Table 3, and are expressed in level of service (LOS) and delay (seconds per vehicle) per lane group and presented in Figure 12. The 95% and 50% queue results for each intersection are also presented in Table 3, and are expressed in feet. The detailed analysis worksheets are included in Appendix C.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the intersections. The capacity analysis results indicate that all movements operate at acceptable LOS under existing conditions with the exception of the following:

- N Quinn Street and Wilson Boulevard
 - Southbound right lane (AM peak hour)
- N Oak Street and Key Boulevard
 - Northbound left/right lane (AM peak hour)
- N Oak Street and Wilson Boulevard
 - Northbound left/thru lane (AM peak hour)
- N Nash Street (east) and Key Boulevard (AM peak hour)
 - Eastbound left lane (AM and PM peak hour)
- N Nash Street and Lee Highway
 - Southbound left lane (AM peak hour)

None of the movements listed above cause the intersections to operate below acceptable LOS thresholds with the exception of the intersection of N Nash Street (east) and Key Boulevard in the AM peak hour. Signalization as mitigation at this intersection is considered in the signal warrant analysis section.

Table 3: Existing Capacity Analysis

			Existing 2016											
	Intersection (Movement)	Storage		AN	/I Peak			PIV	l Peak			Saturd	ay Pea	k
	intersection (Movement)	Length (ft)	LOS Delay		Queue 50th 95th		LOS Delay		Queue 50th 95th		LOS Delay		Queue 50th 95th	
1	N Quinn Street and Key Boulevard				301	.11 55111			301	1 33111			30(1)	331
_	Overall Intersection (Signalized)		С	32.4			В	15.5			В	13.6		
	Eastbound Left/Thru/Right	500	c	31.0	77	132	В	15.6	13	35	c	26.3	19	5
	Westbound Left/Thru/Right	315	C	32.9	82	143	В	19.6	106	164	c	28.7	55	9
	Northbound Left/Thru	200	В	17.6		98	В	13.0	70	194	A	5.1	28	2
	•				60									
	Northbound Right	50	A	2.3	0	4	Α	3.7	2	m9	A	1.5	0	m
	Southbound Left/Thru	525	D	39.1	340	#550	В	17.2	56	99	Α	8.3	30	5
	Southbound Right	75	Α	7.6	0	7	В	14.9	0	11	Α	7.6	0	1
2	N Quinn Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	2.8			Α	1.2			Α	0.6		
	Westbound Left/Right	315	В	12.3		5	В	11.8		8	В	10.7		:
	Northbound Thru/Right	375	Α	0.0		0	Α	0.0		0	Α	0.0		(
	Southbound Left/Thru	200	Α	8.2		13	Α	8.0		0	Α	7.7		(
3	N Quinn Street and Wilson Boulevard													
	Overall Intersection (Signalized)		С	34.4			В	14.6			С	21.3		
	Westbound Thru	325	В	10.0	60	61	Α	9.1	100	76	В	18.2	111	1
	Westbound Right	85	С	23.8	9	28	В	18.1	10	18	С	34.7	39	3
	Southbound Right	375	F	93.8	9	m15	С	29.3	44	107	В	14.5	79	1
	N Pierce Street and Wilson Boulevard						_							
•	Overall Intersection (Signalized)		В	12.5			В	17.8			В	19.8		
	Westbound Left/Thru	675	A	3.9	41	58	В	13.6	245	317	В	12.3	123	1
	Northbound Left	250	D	37.7	96	162	D	36.2	108	161	D	40.0	117	1
_		230	10	37.7	90	102	U	30.2	100	101	U	40.0	11/	
5	N Pierce Street and Clarendon Boulevard		_				_							
	Overall Intersection (Signalized)		В	13.6			В	18.4			Α	8.6		
	Eastbound Left/Thru	500	Α	8.1	189	339	Α	9.1	115	154	Α	3.6	58	ç
	Eastbound Thru/Right	500	Α	8.1	189	339	Α	9.1	115	154	Α	3.6	58	ç
	Northbound Thru/Right	185	D	39.5	92	147	С	26.6	31	66	D	36.6	20	4
	Southbound Left/Thru	250	D	41.1	24	64	D	41.1	160	#271	С	30.6	21	4
3	N Oak Street and Key Boulevard													
	Overall Intersection (Unsignalized)		В	13.6			Α	5.5			Α	4.4		
	Eastbound Thru/Right	285	Α	0.0		0	Α	0.0		0	Α	0.0		(
	Westbound Left/Thru	150	В	10.8		8	Α	8.1		5	Α	7.8		:
	Northbound Left/Right	200	F	65.1		165	С	16.0		35	В	11.2		1
)	N Oak Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	5.7			Α	3.8			Α	2.2		
	Eastbound Left/Right	850	В	13.1		33	В	10.8		10	В	10.0		
	Northbound Left/Thru	200	A	7.8		8	A	7.7		3	A	7.7		
	Southbound Thru/Right	200	A	0.0		0	A	0.0		0	A	0.0		
_	. 0	200	A	0.0		U	А	0.0		U	A	0.0		
)	N Oak Street and Wilson Boulevard		_				_				_			
	Overall Intersection (Signalized)		С	30.2			С	25.9			С	23.2		
	Westbound Thru	125	В	19.7	106	130	В	19.1	109	138	В	16.0	56	8
	Westbound Thru/Right	75	В	19.7	106	130	В	19.1	109	138	В	16.0	56	8
	Northbound Left/Thru	25	Е	58.4	147	m#228	D	50.2	116	176	D	47.6	64	1.
	Southbound Thru/Right	200	С	34.9	95	m144	С	29.0	50	93	С	25.7	19	5
L	N Oak Street and Clarendon Boulevard													
	Overall Intersection (Signalized)		В	14.6			В	14.4			В	15.6		
	Eastbound Left	75	D	52.7	78	m#171	D	47.7	70	m119	D	51.1	49	9
	Eastbound Thru/Right	550	Α	6.2	66	64	Α	6.9	64	m90	Α	7.3	45	5
	Northbound Thru/Right	825	С	29.2	45	88	С	28.2	33	72	С	27.3	20	
	Southbound Left/Thru	25	Α	8.1	8	10	A	7.0	8	9	В	11.4	9	1
	N Nash Street and Key Boulevard		Ė								Ė			
•	Overall Intersection (Unsignalized)		Α	3.1			Α	4.4			Α	3.4		
	Eastbound Left/Thru	135	A	8.5		15	A	8.5		6	A	7.7		
	Westbound Thru/Right					0		8.5 0.0			A	0.0		
		65 650	A	0.0			A			0				
	Southbound Left/Right	650	С	19.0		20	С	15.7		35	В	10.3		
}	N Nash Street (east) and Key Boulevard		1				Ī	_				_		
	Overall Intersection (Unsignalized)		F	102.1			Α	8.8			Α	3.6		
	Eastbound Left	65	F	351.4		658	F	50.4		105	В	12.3		1
	Eastbound Thru/Right	65	С	19.7		53	В	14.7		18	В	10.5		
	Westbound Left/Thru/Right	25	Α	0.0		0	Α	0.0		0	В	11.2		(

			Existing 2016											
	Interception (Bases and)	Storage		AM Peak PM Peak							Saturday Peak			
	Intersection (Movement)	Length (ft)	LOS Delay		Queue 50th 95th		LOS Delay		Queue 50th 95th		LOS Delay		Queue 50th 95th	
	Northbound Left/Thru	400	Α	9.0		5	Α	8.7		10	Α	7.8		3
	Northbound Thru/Right	400	Α	0.2		0	Α	0.3		0	Α	0.1		0
	Southbound Left/Thru	150	Α	8.7		0	Α	8.6		0	Α	7.6		0
	Southbound Right	150	Α	0.0		0	Α	0.0		0	Α	0.0		0
14	N Nash Street and Lee Highway													
	Overall Intersection (Signalized)		В	16.2			В	11.8			Α	9.1		
	Eastbound Left/Thru	1450	Α	6.9	113	172	Α	4.8	66	113	Α	3.3	49	79
	Eastbound Thru	1450	Α	6.9	113	172	Α	4.8	66	113	Α	3.3	49	79
	Eastbound Thru/Right	1450	Α	6.9	113	172	Α	4.8	66	113	Α	3.3	49	79
	Northbound Thru/Right	650	D	45.7	103	m134	С	34.1	28	76	D	45.3	14	51
	Southbound Left	125	Ε	64.9	65	111	D	46.6	50	87	D	42.5	36	69
	Southbound Thru	125	С	30.6	17	36	С	33.5	14	32	D	37.1	8	24
15	N Fort Myer Drive and Lee Highway NB													
	Overall Intersection (Signalized)		В	17.5			В	17.9			В	17.1		
	Eastbound Thru	225	В	10.8	129	136	С	20.1	180	126	Α	9.0	52	66
	Eastbound Right	70	Α	7.4	18	29	В	17.8	49	79	Α	8.2	14	44
	Southbound Left	250	С	31.9	134	239	В	18.0	141	219	С	24.4	46	122
	Southbound Left/Thru	250	С	26.3	154	198	В	16.0	160	188	С	25.1	121	161

m - Volume for 95th percentile queue is metered by upstream signal

^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

Traffic Impact Study and Transportation Management Plan – West Rosslyn

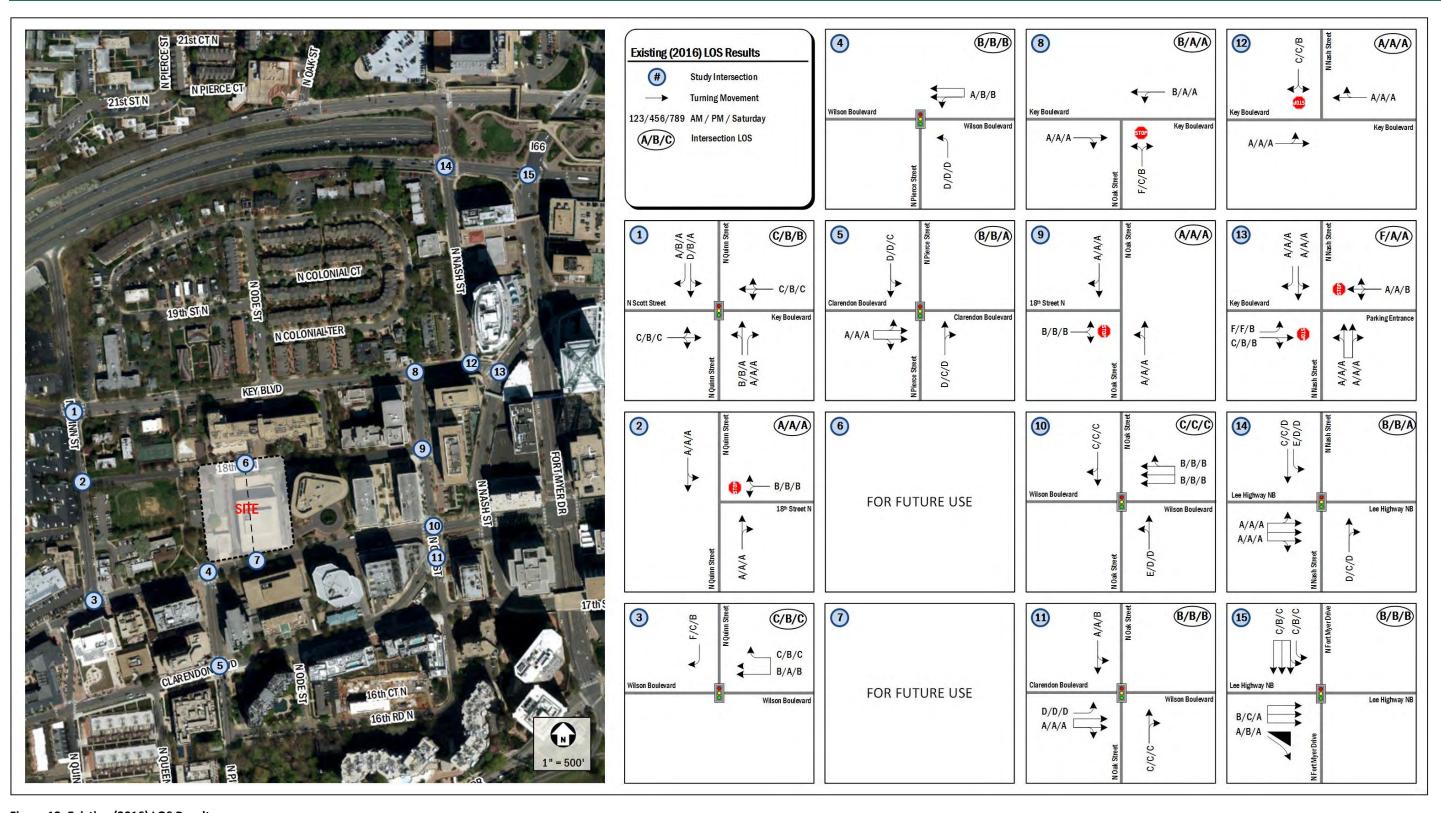


Figure 12: Existing (2016) LOS Results

FUTURE CONDITIONS WITHOUT DEVELOPMENT (2019)

Planned Transportation Network Improvements

Three projects are planned in the vicinity of the subject site in the near future may impact some of the study area intersections. As stated in the *Rosslyn Sector Plan* adopted in July 2015 by the County of Arlington, the county plans to extend 18th Street east from N Quinn Street in order to create a central spine and link together destinations such as the Metro Station and Potomac River across Rosslyn. The 18th Street Corridor will offer a protected pedestrian environment with amenities typical found along a street frontage.

N Fort Myer Drive is planned to undergo a transformation with the current one-way configuration being converted into a two-way street. Included in this transformation is the removal of the N Fort Myer Drive tunnel that currently runs under Wilson Boulevard. It is thought that these improvements will enhance the walkablility of these streets. The timeline and funding for these two improvement projects are unknown at this time; therefore, not included in this study.

A planned improvement included in the Rosslyn Sector plan is included with the opening of the West Rosslyn development. A new street will bisect the development into an eastern and western parcel and create a connection between 18th Street N and Wilson Boulevard.

Future without Development Traffic Volumes

The proposed mixed use development is anticipated to be completed in 2019. According to historical data provided by VDOT, there has been negative or no growth along streets within the study areas in recent years. As a conservative measure, a regional growth rate of 0.5% annually has been applied for 2019 and 2025 scenarios. Additional traffic volumes added to the network from the background growth rate are presented in Figure 14.

In addition to the regional background growth, ten developments in the vicinity of the site were taken into consideration. As discussed at the scoping meeting, the following ten developments were included in the analysis and their locations relative to West Rosslyn can be found in Figure 13:

- Rosslyn Gateway: The site is bounded by Lee Highway to the north, N Moore Street to the east, 19th Street N to the south and N Fort Myer Drive to the west. It is a mixed-use development that will consist of approximately 500,000 square feet of office space, a hotel with 148 rooms, 273 multi-family units, and 26,000 square feet of retail. The project is expected to generate 536 weekday AM peak hour vehicle trips, 647 PM peak hour vehicle trips, and 455 Saturday peak hour vehicle trips.
- Central Place: Central Place is located adjacent to N Moore Street and N Lynn Street and bordered by 19th Street N on the north. The mixed-used development consists of 350 condominium units, 45,000 square feet of retail, and approximately 570,000 square feet of office space. The project is expected to generate 444 weekday AM peak hour vehicle trips, 463 PM peak hour vehicle trips, and 233 Saturday peak hour vehicle.
- Rosslyn Plaza: Rosslyn Plaza is located between N. Kent Street and N. Arlington Ridge Road in the southeast corner of Rosslyn. Phase one will be completed by 2018 and full build-out for the site was anticipated for 2021 and will include 1,900,000 square feet of office space, 50,000 square feet of retail space, 300 hotel rooms, and 600 multifamily units. The project is expected to generate 556 net weekday AM peak hour vehicle trips, 678 net weekday PM peak hour vehicle trips, and 471 net Saturday peak hour vehicle trips based on the Traffic Impact Study prepared by Wells & Associates dated December 24, 2015.

- 1411 Key Boulevard: 1411 Key Boulevard is located in on the north side of Key Boulevard, west of N Nash Street. The project will redevelop existing property that consists of 32 rental apartments and one single-family residence into 63 residential dwelling units that include 57 residential condominiums and six townhomes. With an expected build-out date of 2019, the project is expected to generate 20 net weekday AM peak hour vehicle trips, 24 net weekday PM peak hour vehicle trips, and 31 Saturday peak hour vehicle trips based on the Traffic Impact Analysis Report prepared by Wells & Associates dated October 6, 2014.
- 1401 Wilson Boulevard: The site is by bounded by Key Boulevard on the north, Wilson Boulevard by the south, and by N Oak Street on the west. The current two 12-story buildings on the property would be razed to create a mixed-use development with over 500,000 square feet of office space, a 40,000 square foot grocery store, 18,000 square feet of retail, and 288 residential units. It is expected to be open by 2019. The project is expected to generate 353 net weekday AM peak hour vehicle trips, 485 weekday PM peak hour vehicle trips, and 430 Saturday peak hour vehicle trips.
- Colony House: The site is located in at the southwest corner of Lee Highway and N Quinn Street (1700 Lee Highway). It will replace a 31,644 square foot furniture store with a 168-room all-suites, extended-stay hotel. The project is expected to generate 38 net weekday AM peak hour vehicle trips, 40 net weekday PM peak hour vehicle trips, and 73 net Saturday peak hour vehicle trips based on the Traffic Impact Study prepared by Gorove/Slade Associates dated January 31, 2012.
- Pierce Queen Apartments: The development is located in Rosslyn bound by 15th Street N, Fairfax Drive, N Pierce Street, and N Queen Street. It is expected to be open by late 2016 and will consist of 196 residential units. The project is expected to generate 61 weekday AM peak hour vehicle trips and 77 weekday PM peak hour vehicle trips based on the Traffic Impact Analysis Report prepared by Wells & Associates dated July 13, 2012.
- APAH Queens Court: The development is located on N Quinn Street between 18th Street N and Key Boulevard. This project will see the redevelopment of an existing affordable housing complex containing 39 residential units into a larger residential property containing 251 residential units. It is expected to be open by 2020 and will consist of 251 residential units. The project is expected to generate 28 net weekday AM peak hour vehicle trips, 23 net weekday PM peak hour vehicle trips, and 14 net Saturday peak hour vehicle trips based on the Scoping Form prepared by Gorove/Slade Associates dated March 1, 2016.
- **1812 N Moore Street**: This development is located along N Moore Street between 19th Street N and Wilson Boulevard. It will consist of 570,000 square feet of office space and 11,000 square feet of retail. The project is expected to generate 382 weekday AM peak hour vehicle trips, 372 weekday PM peak hour vehicle trips, and 144 Saturday peak hour vehicle trips.
- Wilson School: The school, located at 1601 Wilson Boulevard will be razed and redeveloped into a larger school serving more students. The school will serve approximately 950 students from grades 6-12, along with visitors and staff. The project is expected to generate 364 weekday AM peak hour vehicle trips, 295 weekday PM peak hour vehicle trips, and 44 Saturday peak hour vehicle trips based on the Trip Generation and Trip assignment prepared by Toole Design Group dated March 30, 2016. Due to the Wilson School's PM peak hour falling outside of the PM study peak hour, PM school trips were reduced to 60%.

The total traffic generated by the background developments are presented in Figure 15. Diagrams showing the trips generated by each background development are presented in Appendix D.

Trips generated by the approved background developments were added to the existing traffic volume with approved growth rates in order to generate future conditions without development (2019) traffic volumes. The Future without Development (2019) traffic volumes are shown in Figure 16.

Table 4: Traffic Generated by Background Developments

						Weekd	ау			\	Veekend	
Land Use	ITE Code	Size	AN	1 Peak Ho	our		PM Peak H	our	Daily	Sat Peak	Hour	Daily
			In	Out	Total	In	Out	Total	Total	In	Out	Total
Rosslyn Gateway (1)												
Total New Vehicle-Trips			407	129	536	213	434	647		240	215	455
Central Place (2)												
Total New Vehicle-Trips			348	96	444	124	339	463		125	108	233
Rosslyn Plaza (3)												
Total Development Trips			1,992	585	2,577	889	2,245	3,134		985	868	1853
Total New External Trips			388	168	556	232	446	678		263	208	471
1411 Key Boulevard (4)												
Net New Residential Site Trips			4	17	21	17	8	20	249	19	12	31
1401 Wilson Boulevard and 140	00 Key Boulevard ⁽⁵⁾											
Total Net New Vehicle-Trips			266	87	353	150	335	485		223	207	430
Colony House (6)												
Total Trips			21	16	38	18	22	40	494	41	32	73
Pierce Queen Apartments (7)												
Total Trips			12	49	61	50	27	77	803	31	30	61
APAH Queens Court (8)												
Proposed Residential												
Net New Trips			5	23	28	0 15	8	23	298	6	8	14
1812 North Moore Street (9)												
Total New Vehicle-Trips			335	47	382	68	304	372		77	67	144
Wilson School (10)												
Total			227	137	364	104	191	295		27	17	44
Total Background Trips			2,013	769	2,783	991	2,114	3,100	1,844	1,052	909	1,941

(1), (2), (5), (9): Extracted from Rosslyn Plaza TIA (Updated TIA Submission 12.24.15) prepared by Wells + Associates

(6) Extracted from 1700 Lee Highway (1.31.12) TIA prepared by Gorove Slade Associates

^{(7):} Extracted from Pierce Queen Apartments TIA (07.13.2012) prepared by Wells + Associates

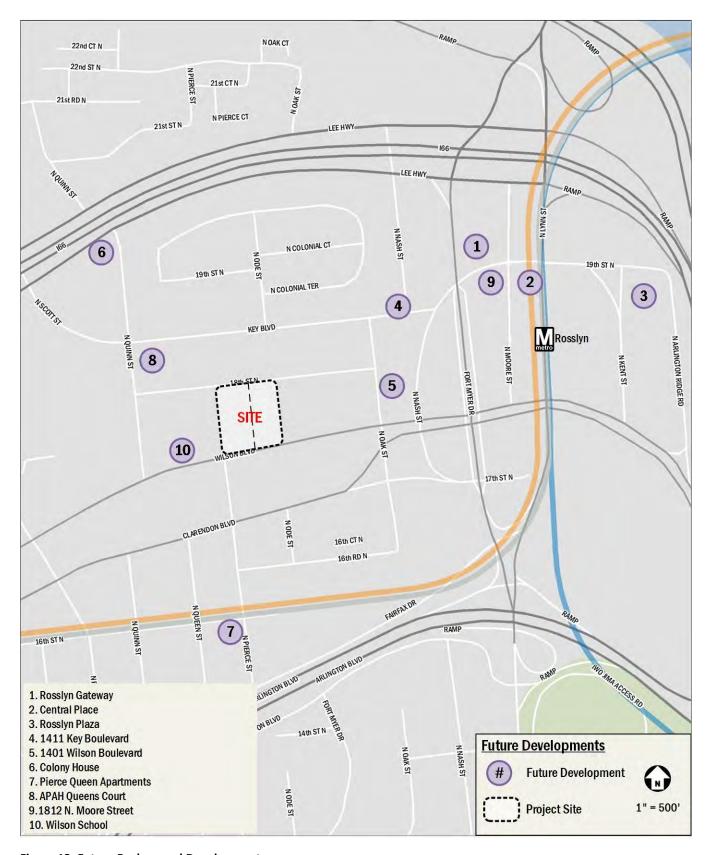


Figure 13: Future Background Developments

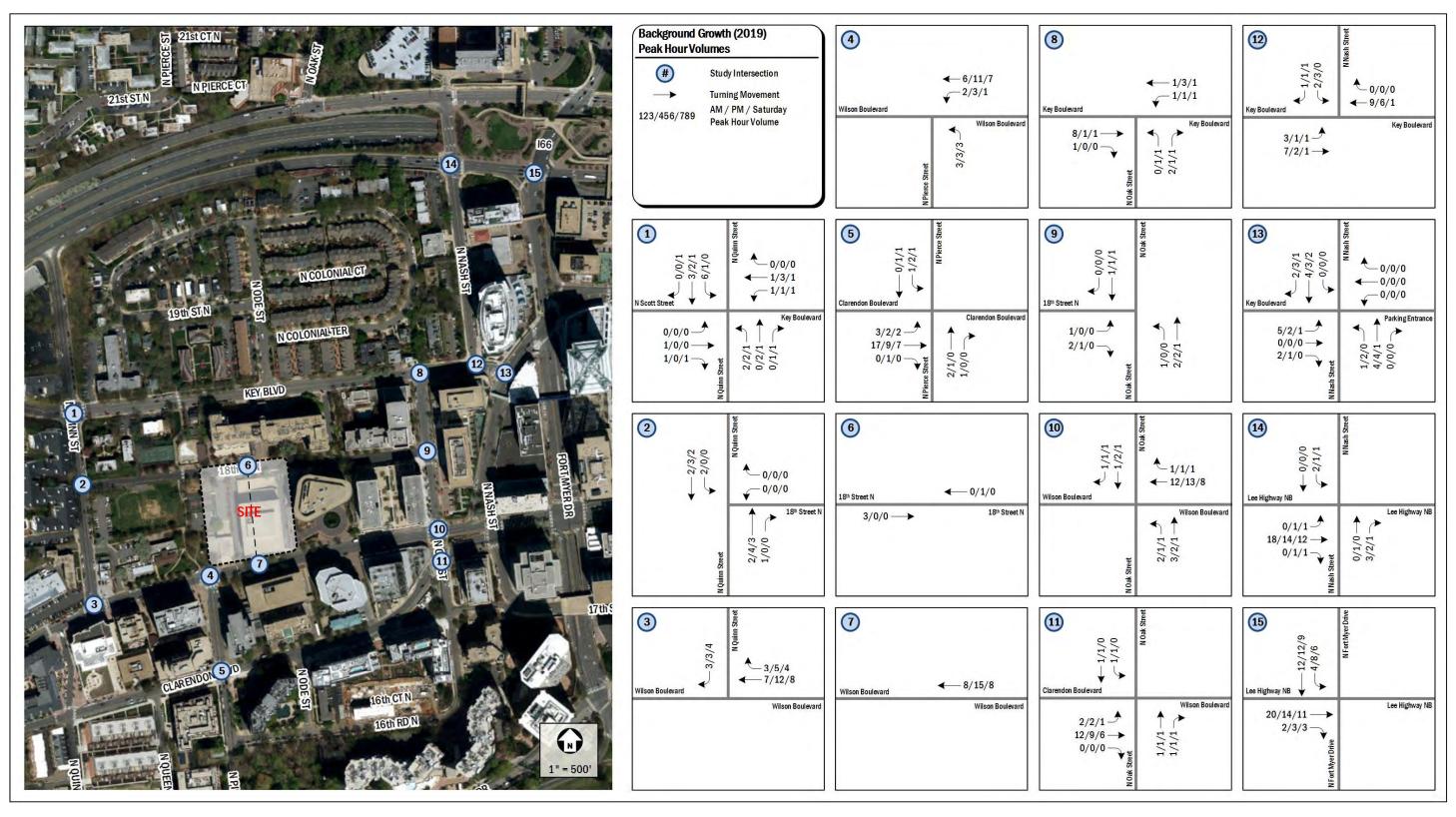


Figure 14: Background Growth (2019) Traffic Volumes

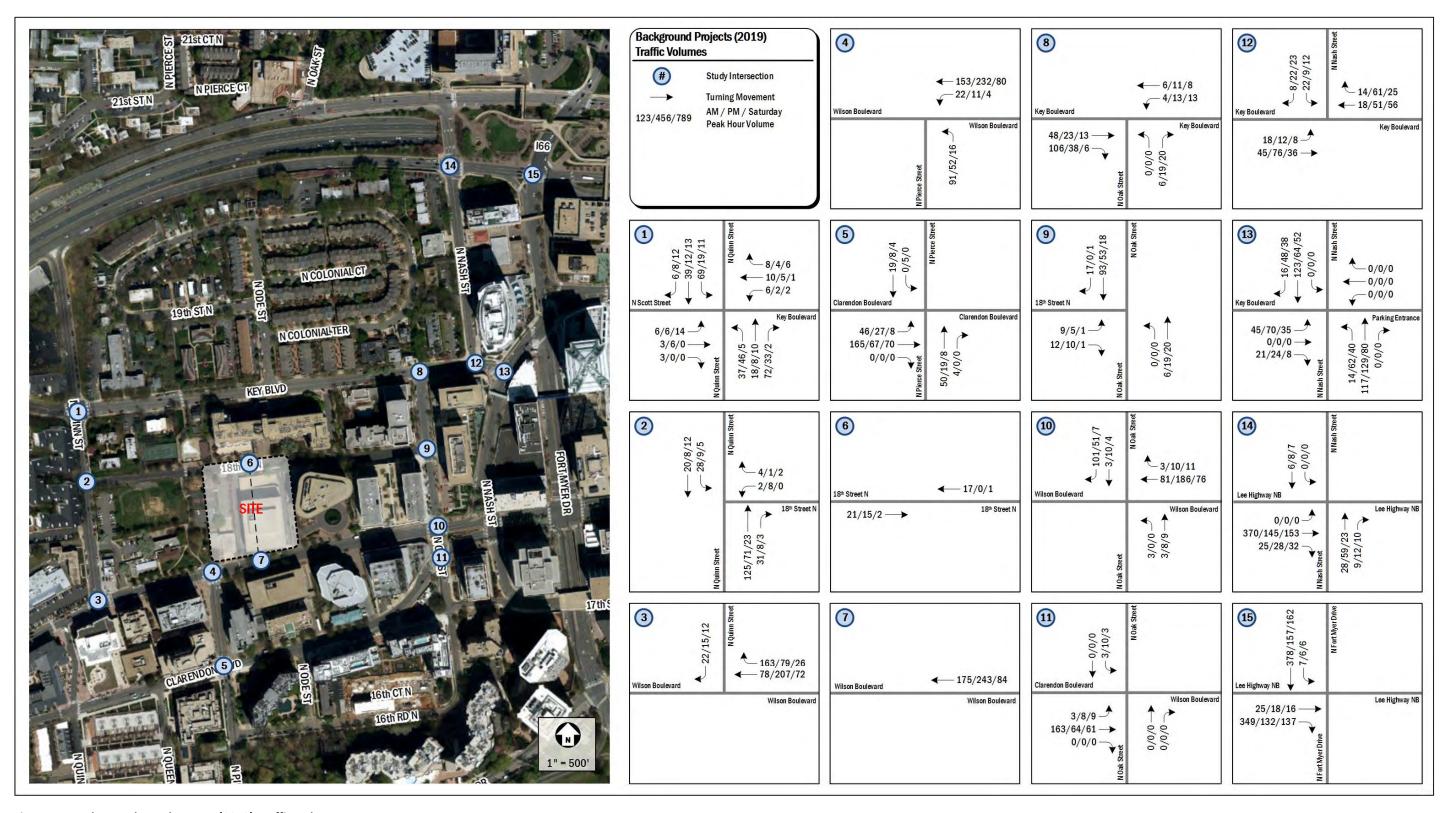


Figure 15: Background Development (2019) Traffic Volumes

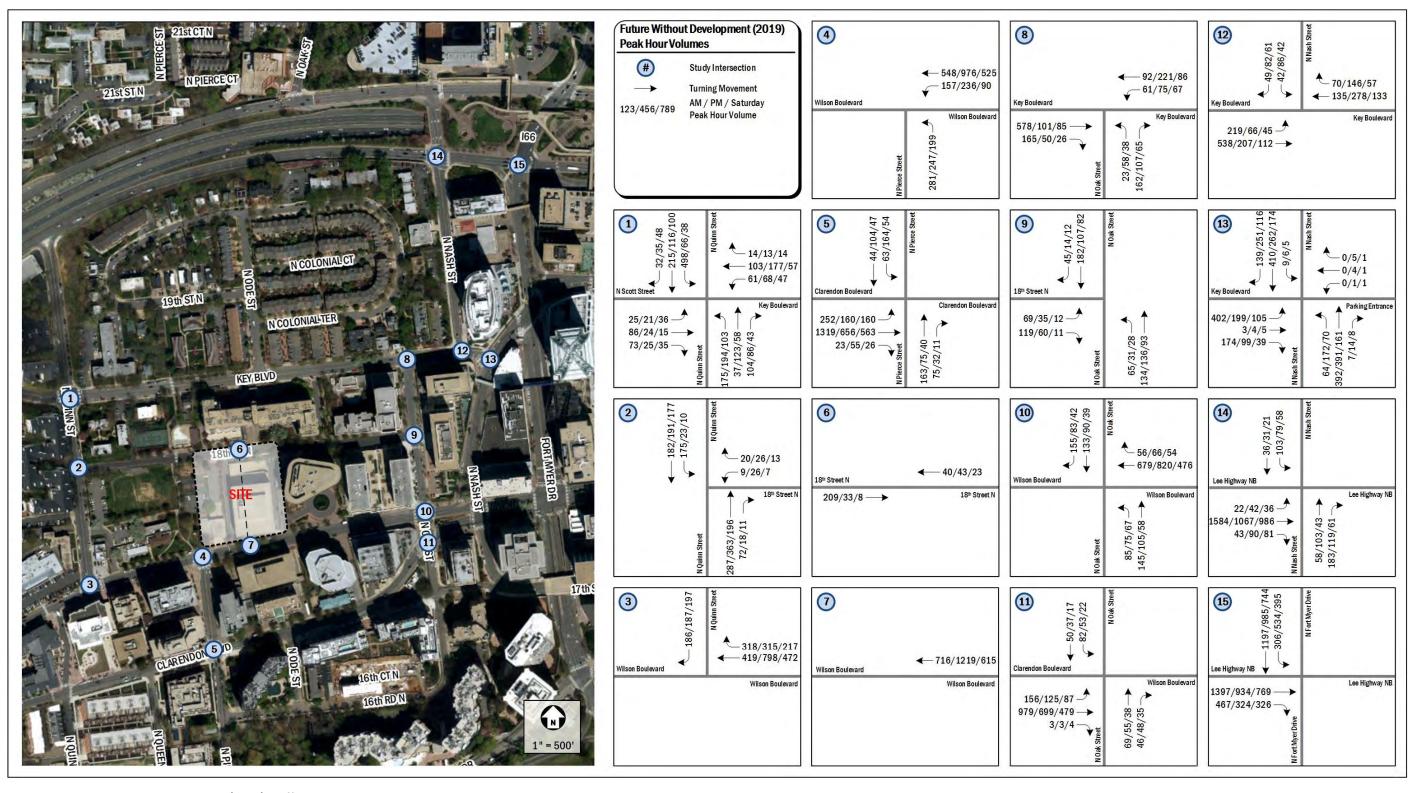


Figure 16: Future without Development (2019) Traffic Volumes

Future without Development Capacity Analysis

Capacity analyses were performed at the intersections within the study area during the weekday AM, weekday PM, and Saturday peak hours for the future conditions without development scenario. *Synchro, Version 9.1* was used to analyze the study intersections based on the <u>Highway Capacity Manual</u> (HCM) methodology and include level of service, delay, and queue length comparisons for the turning movements analyzed.

Peak hour factors were applied in accordance with *Traffic Impact Analysis Regulations Administrative Guidelines* prepared by VDOT dated July 2012. As such, peak hour factors by approach between 0.92 and 1.00 were used for all future scenarios. Where the calculated peak hour factor based on the existing turning movement counts was greater than 0.92, the calculated factor was applied. Where the calculated factor was 0.92 or less, a factor of 0.92 was applied.

The results of the intersection capacity analysis are presented in Table 5, and are expressed in level of service (LOS) and delay (seconds per vehicle) per lane group. The 95% and 50% queue results for each intersection are also presented in Table 5, and are expressed in feet. The detailed analysis worksheets are contained in Appendix E.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the intersections. All movements at the study intersections operate at acceptable levels of service consistent with the Existing Conditions scenario, with the exception of the following:

- N Quinn Street and Key Boulevard
 - Southbound left/thru lane (AM peak hour)
- N Pierce Street and Clarendon Boulevard
 - Southbound left/thru lane (AM peak hour)
- N Oak Street and Clarendon Boulevard
 - Eastbound left lane (AM peak hour)
- N Nash Street (east) and Key Boulevard (PM peak hour)
- N Nash Street and Lee Highway
 - Southbound left lane (PM peak hour)

None of the movements listed above cause the intersections to operate below acceptable LOS thresholds with the exception of the intersection of N Nash Street (east) and Key Boulevard in the PM peak hour. Signalization as mitigation at this intersection is considered in the signal warrant analysis section.

March 29, 2016 37

Table 5: Future without Development (2019) Capacity Results

						Future	e Wit			ent (201	9)			
	Intersection (Movement)	Storage		ΑI	VI Peak			PIV	l Peak			Saturd	ay Pea	k
	intersection (Movement)	Length	LO	S Delay		ueue h 95th	LOS	S Delay		ueue h 95th	LOS	Delay		eue 95th
1	N Quinn Street and Key Boulevard													
	Overall Intersection (Signalized)		D	54.1			В	16.9			В	13.6		
	Eastbound Left/Thru/Right	500	С	31.3	77	144	В	15.7	16	43	С	26.8	25	64
	Westbound Left/Thru/Right	315	С	34.6	92	162	В	19.6	106	172	С	28.8	55	10
	Northbound Left/Thru	200	D	49.6	85	#274	В	17.0	91	195	Α	5.0	28	3:
	Northbound Right	50	Α	1.8	0	5	Α	6.7	3	m18	Α	1.5	0	m
	Southbound Left/Thru	525	Ε	75.9	~497	717	В	18.6	72	125	Α	8.5	35	6
	Southbound Right	75	Α	7.6	0	10	В	15.0	0	16	Α	7.7	0	1
2	N Quinn Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	2.6			Α	1.4			Α	0.7		
	Westbound Left/Right	315	В	14.8		8	В	13.4		10	В	10.7		3
	Northbound Thru/Right	375	Α	0.0		0	Α	0.0		0	Α	0.0		C
	Southbound Left/Thru	200	Α	8.8		15	Α	8.3		3	Α	7.8		(
3	N Quinn Street and Wilson Boulevard													
-	Overall Intersection (Signalized)		С	23.4			В	12.0			С	21.2		
	Westbound Thru	325	A	9.5	56	79	A	8.1	102	100	В	18.5	130	11
	Westbound Right	85	c	29.4	28	m50	В	11.3	8	12	C	33.7	22	3
	Southbound Right	375	D	44.4	15	m22	C	30.0	69	138	В	13.8	81	1
4	N Pierce Street and Wilson Boulevard	373	+			11122	_	30.0		130		13.0		
~	Overall Intersection (Signalized)		В	16.1			С	20.9			В	19.9		
	Westbound Left/Thru	675	A	6.1	77	m97	В	17.2	335	428	В	13.4	154	2:
	Northbound Left	250	D	41.8	151	m#228	D	38.0	130	210	D	40.2	120	18
5	N Pierce Street and Clarendon Boulevard	230	-	41.0	131	111#220	U	36.0	130	210	U	40.2	120	10
5			В	10.2			В	171				0.1		
	Overall Intersection (Signalized)	500		18.2	200	400	В	17.1	420	400	A	9.1		
	Eastbound Left/Thru	500	В	12.0	300	482	Α	9.5	138	183	Α	3.8	66	1
	Eastbound Thru/Right	500	В	12.0	300	482	Α	9.5	138	183	Α	3.8	66	1:
	Northbound Thru/Right	185	D	41.9	126	195	С	27.1	41	85	D	36.5	22	5
	Southbound Left/Thru	250	Е	55.6	39	#142	D	37.3	125	#280	С	34.4	22	5
8	N Oak Street and Key Boulevard													
	Overall Intersection (Unsignalized)		В	14.3			Α	5.7			Α	4.6		
	Eastbound Thru/Right	285	Α	0.0		0	Α	0.0		0	Α	0.0		(
	Westbound Left/Thru	150	В	11.7		10	Α	8.3		5	Α	7.8		į
	Northbound Left/Right	200	F	79.8		183	С	17.4		45	В	11.4		1
9	N Oak Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	5.6			Α	3.5			Α	1.9		
	Eastbound Left/Right	850	С	15.3		43	В	11.4		15	В	10.1		3
	Northbound Left/Thru	200	Α	8.1		5	Α	7.8		3	Α	7.7		3
	Southbound Thru/Right	200	Α	0.0		0	Α	0.0		0	Α	0.0		(
0	N Oak Street and Wilson Boulevard													
	Overall Intersection (Signalized)		D	46.0			С	26.8			С	23.2		
	Westbound Thru	125	В	20.0	115	150	С	20.5	145	184	В	16.5	71	10
	Westbound Thru/Right	75	В	20.0	115	150	С	20.5	145	184	В	16.5	71	10
	Northbound Left/Thru	25	F	133.2	~169	m#286	D	53.6	116	188	D	49.3	70	13
	Southbound Thru/Right	200	D	42.7	136	m200	C	30.6	67	132	C	25.6	19	5
11	N Oak Street and Clarendon Boulevard	200	Ť	,		200		00.0		102				
	Overall Intersection (Signalized)		В	14.1			В	15.1			В	15.6		
	Eastbound Left	75	E	56.5	85	m126	D	48.4	77	m130	D	52.7	53	10
	Eastbound Thru/Right	550	A	6.0	60	91	A	8.0	76	m122	A	7.4	50	6
	Northbound Thru/Right	825	C	29.1	45	93	C	28.0	30	73	C	27.1	18	5
	. 5	25	В	10.8	43 14		A	9.3	12	75 15	В			
_	Southbound Left/Thru	25	В	10.8	14	m21	А	9.3	12	15	В	11.9	10	1
2	N Nash Street and Key Boulevard		1.								۱.			
	Overall Intersection (Unsignalized)	46-	A	4.5		4.5	Α	4.9		_	Α	3.4		
	Eastbound Left/Thru	135	Α	8.6		18	Α	8.9		5	Α	7.9		
	Westbound Thru/Right	65	Α	0.0		0	Α	0.0		0	Α	0.0		
	Southbound Left/Right	650	D	30.7		48	С	21.1		58	В	11.5		1
L3	N Nash Street (east) and Key Boulevard													
	Overall Intersection (Unsignalized)		F	189.0			F	61.3			Α	4.2		
		CE	F	738.5		933	F	414.1		408	С	16.6		2
	Eastbound Left	65	F	730.3		500		12 112			_	10.0		
	Eastbound Left Eastbound Thru/Right	65	D	25.9		75	C	17.0		28	В	11.2		ļ

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						Futur	e Wit	hout De	velopm	ent (201	9)			
	Interception (Marromont)	Storage		ΑI	M Peak			PIV	1 Peak			Saturd	ay Pea	k
	Intersection (Movement)	Length	LOS	6 Delay		ueue h 95th	LO	S Delay		ieue n 95th	LOS	Delay		eue 95th
	Northbound Left/Thru	400	Α	9.5		8	Α	9.2		18	Α	8.0		5
	Northbound Thru/Right	400	Α	0.4		0	Α	0.6		0	Α	0.1		0
	Southbound Left/Thru	150	Α	9.0		0	Α	9.0		0	Α	7.8		0
	Southbound Right	150	Α	0.0		0	Α	0.0		0	Α	0.0		0
14	N Nash Street and Lee Highway													
	Overall Intersection (Signalized)		В	16.2			В	14.3			Α	9.2		
	Eastbound Left/Thru	1450	Α	8.9	178	250	Α	6.2	92	154	Α	3.5	63	100
	Eastbound Thru	1450	Α	8.9	178	250	Α	6.2	92	154	Α	3.5	63	100
	Eastbound Thru/Right	1450	Α	8.9	178	250	Α	6.2	92	154	Α	3.5	63	100
	Northbound Thru/Right	650	D	46.9	135	m174	D	40.2	94	154	D	44.2	26	74
	Southbound Left	125	E	56.6	60	#131	Ε	56.4	47	92	D	42.2	34	71
	Southbound Thru	125	С	29.2	18	42	С	31.6	17	38	D	37.4	12	33
15	N Fort Myer Drive and Lee Highway													
	Overall Intersection (Signalized)		С	20.2			В	18.0			В	18.3		
	Eastbound Thru	225	Α	9.4	115	125	В	18.2	116	122	Α	9.1	52	66
	Eastbound Right	70	В	11.8	96	130	С	21.9	86	125	В	10.8	53	80
	Southbound Left	250	С	32.9	143	251	В	18.7	156	260	С	26.5	77	170
	Southbound Left/Thru	250	С	33.0	258	319	В	16.4	174	216	С	26.6	158	203

m - Volume for 95th percentile queue is metered by upstream signal

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^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

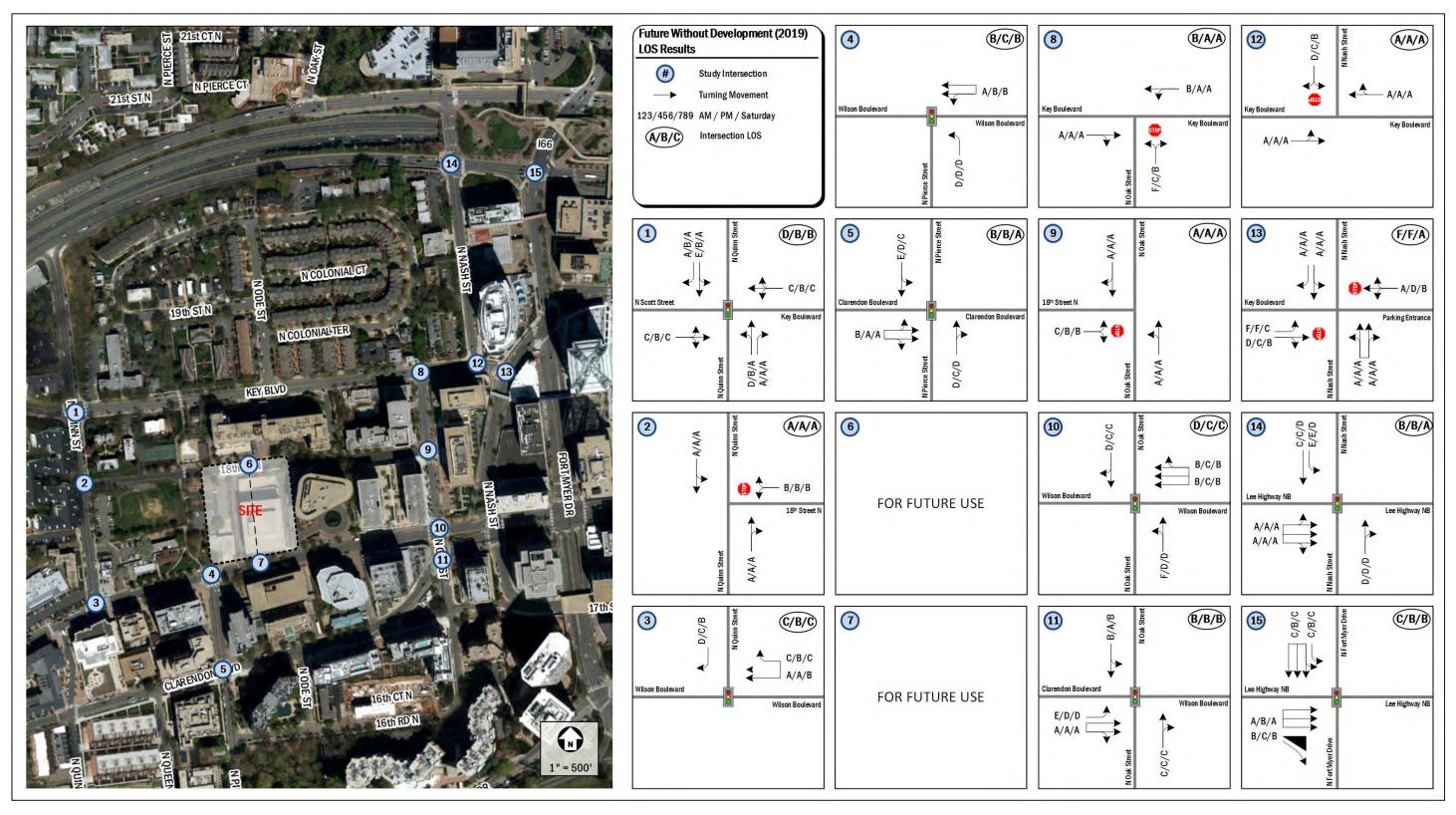


Figure 17: Future without Development (2019) LOS Results

FUTURE CONDITIONS WITH DEVELOPMENT (2019)

The proposed development will consist of approximately 700 residential units, 400,000 square feet of office space, 22,000 square feet of retail, 20,000 square feet of grocery space, a 25,000 square foot reconstruction of Rosslyn Highlands Park, and a redevelopment of the existing fire station. The existing 50,000 square feet office building, existing fire station, and City Park will be replaced by the proposed development.

Site Access

The proposed site is located between 18th Street N and Wilson Boulevard, with the western border of the site near the northern terminus of N Pierce Street. The site location is shown in Figure 1. The proposed site plan for the redevelopment is shown in Figure 5.

Entry to the site on the new street will be accessible from Wilson Boulevard (traveling westbound) and 18th Street N (traveling westbound and eastbound). The proposed western parcel site driveway off the new street will provide direct access to the parking garage as shown on Figure 6. Loading access will be provided to the eastern parcel off of 18th Street N, with western parcel loading available off the new street. The relocation of the fire station to the southeast corner of the site will include a driveway from 18th Street N, in addition to the front entrance along Wilson Boulevard. The roadway configuration for the future with development scenario is shown in Figure 18.

Site Generated Traffic

The <u>Institute of Transportation Engineers (ITE) Trip Generation</u>, 9th Edition, was used to determine the future trips generated by the proposed development.

The site has multiple bus stops surrounding the site and a Metro stations within one-half mile of the site. It is expected that a significant portion of those accessing the site will travel by Metrorail, bus, cab, or on foot/by bicycle during the peak hours, rather than by personal vehicle. Based on data from Table S-3 and Table S-4 of the WMATA Development-Related Ridership Survey (WMATA, 2005), the Arlington County Commercial Building Research Topline Report, and the Arlington County Residential Building Transportation Performance Monitoring study, a TDM/transit mode split reduction of 60% for residential, 40% for office, and 50% of retail was applied to the peak hour vehicular trip generation for the residential land use to reflect the non-auto modes for the proposed development. The retail space planned for the site is envisioned to be primarily neighborhood-serving retail site, justifying the 50% TDM/transit mode split reduction assumed. Table 6 shows the vehicular trips expected to be generated by the proposed project.

Driveway counts were taken at the existing office building, park, and fire station driveways to determine trip generation for the existing uses. The existing office park generated 184 AM peak hour trips, 130 PM peak hour trips, and 26 Saturday peak hour trips. The existing fire station generated 5 AM peak hour trips, 9 PM peak hour trips, and 8 Saturday peak hour trips. The park indicated no peak hour trip generation.

Traffic Impact Study and Transportation Management Plan – West Rosslyn

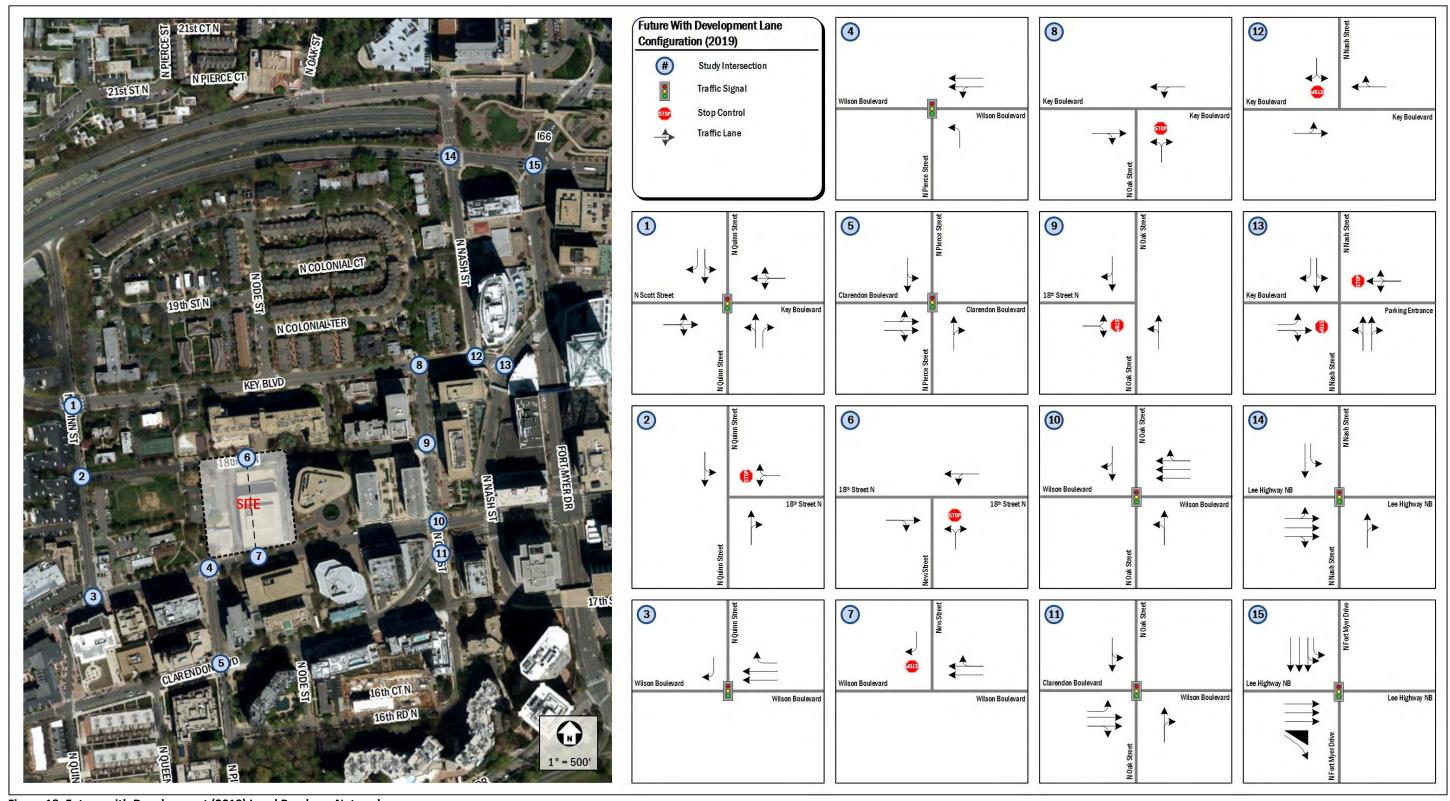


Figure 18: Future with Development (2019) Local Roadway Network

Table 6: Trip Generation

							Weekday					Satu	rday	
Land Use	ITE Code		Size	AN	/I Peak Hou	r	P	M Peak Hou	ır	Daily	S	at Peak Hou	ır	Daily
				In	Out	Total	In	Out	Total	Total	In	Out	Total	Total
Existing Use*														
General Office	710	143	ksf	178	6	184	19	111	130	_	2	24	26	
Specialty Retail	826	14	ksf	178	U	104	13	111	130		2	24	20	
Fire Station	-			3	2	5	6	3	9	-	4	4	8	-
City Park	-	25	ksf	-	-	-	-	-	-	-	-	-	-	-
			Existing Trips	181	8	189	25	114	139	0	6	28	34	0
Proposed Residential														
High-Rise Apartment	222	700	DU	53	157	210	144	92	236	2,800	147	111	258	3,506
Transit Reduction (Residential)**			60%	-32	-94	-126	-86	-56	-142	-1,680	-88	-67	-155	-2,104
Subtotal (Residential vehicle trips)				21	63	84	58	36	94	1,120	59	44	103	1,402
nternal Trip Reduction - (5% AM, 10% F	PM and Sat, 15	% Weekday)		-1	-3	-4	-6	-3	-9	-168	-6	-4	-10	-210
		Total Re	ail External Trips	20	60	80	52	33	85	952	53	40	93	1,192
Proposed Retail														
Fire Station	-			3	2	5	6	3	9	-	4	4	8	-
City Park	-	25	ksf	-	-	-	-	-	-	-	-	-	-	-
Specialty Retail	826	22	ksf	19	18	37	33	41	74	979	55	55	110	925
Transit Reduction (Retail)**			50%	-10	-9	-19	-17	-20	-37	-490	-28	-27	-55	-463
Retial Pass-by Trips			25%	-2	-3	-5	-4	-5	-9	-122	-7	-7	-14	-116
Super Market	850	20	ksf	42	26	68	121	116	237	2,731	184	177	361	3,552
Transit Reduction (Supermarket)***			50%	-21	-13	-34	-61	-58	-119	-1,366	-92	-89	-181	-1,776
Supermarket Pass-by Trips			25%	-5	-4	-9	-15	-15	-30	-341	-23	-22	-45	-444
General Office	710	400	ksf	510	70	580	89	437	526	3,765	93	79	172	844
Transit Reduction (Office) **			40%	-204	-28	-232	-36	-174	-210	-1,506	-37	-32	-69	-338
		Total Re	ail External Trips	332	59	391	116	325	441	3,650	149	138	287	2,184
		Total Site	External Trips	352	119	471	168	358	526	4,602	202	178	380	3,376
			l Pass-by Trips	-7	-7	-14	-19	-20	-39	-463	-30	-29	-59	-560
			Total Site Trips	345	112	457	149	338	487	4,139	172	149	321	2,816
			Net New Trips	164	104	268	124	224	348	4,139	166	121	287	2,816

^{*} Existing trip generation to be based on existing site trips based on intersection counts

The proposed residential and retail development will generate approximately 268 net trips in the AM peak hour, 348 net trips in the PM peak hour, and 287 net trips in the Saturday peak hour.

Site Trip Distribution

The distribution of site trips was based primarily on existing volumes, anticipated traffic patterns and other recent studies conducted in the area. The peak hour trips were calculated and assigned to the roadway network based on the traffic distribution shown in Figure 19. The trips generated by the site are presented in Figure 20. Site Pass-By volumes are presented in Figure 21.

Future with Development Traffic Volumes

The West Rosslyn development provides a new street which will connect the eastern and western parcels of the development while providing a north-south connection between 18th Street N and Wilson Boulevard. In order to determine the traffic volumes on the roadways in the vicinity of the development under the 2019 build condition, the site generated traffic volumes were added to the Future without Development (2019) peak hour traffic volumes. The traffic volumes for the Future with Development (2019) conditions are presented in Figure 22.

^{**}Table S-3 and S-4 from WMATA Survey, Arlington County Commercial Building Research Topline Report, and Arlington County Residential Building Transportation Performance Monitoring Study

^{***}Final Mode split based on consultation with Arlington County and review of comparable sites

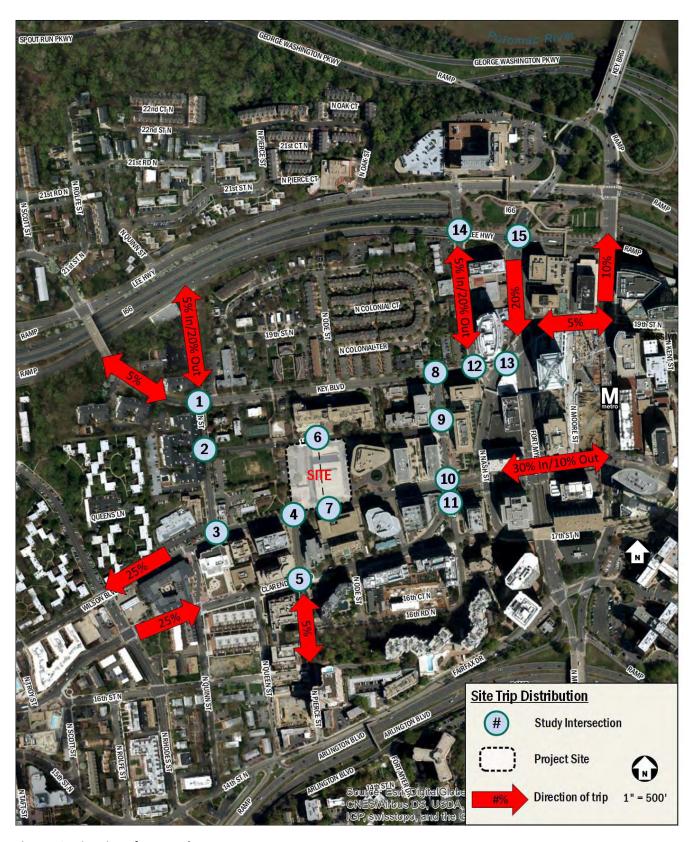


Figure 19: Direction of Approach

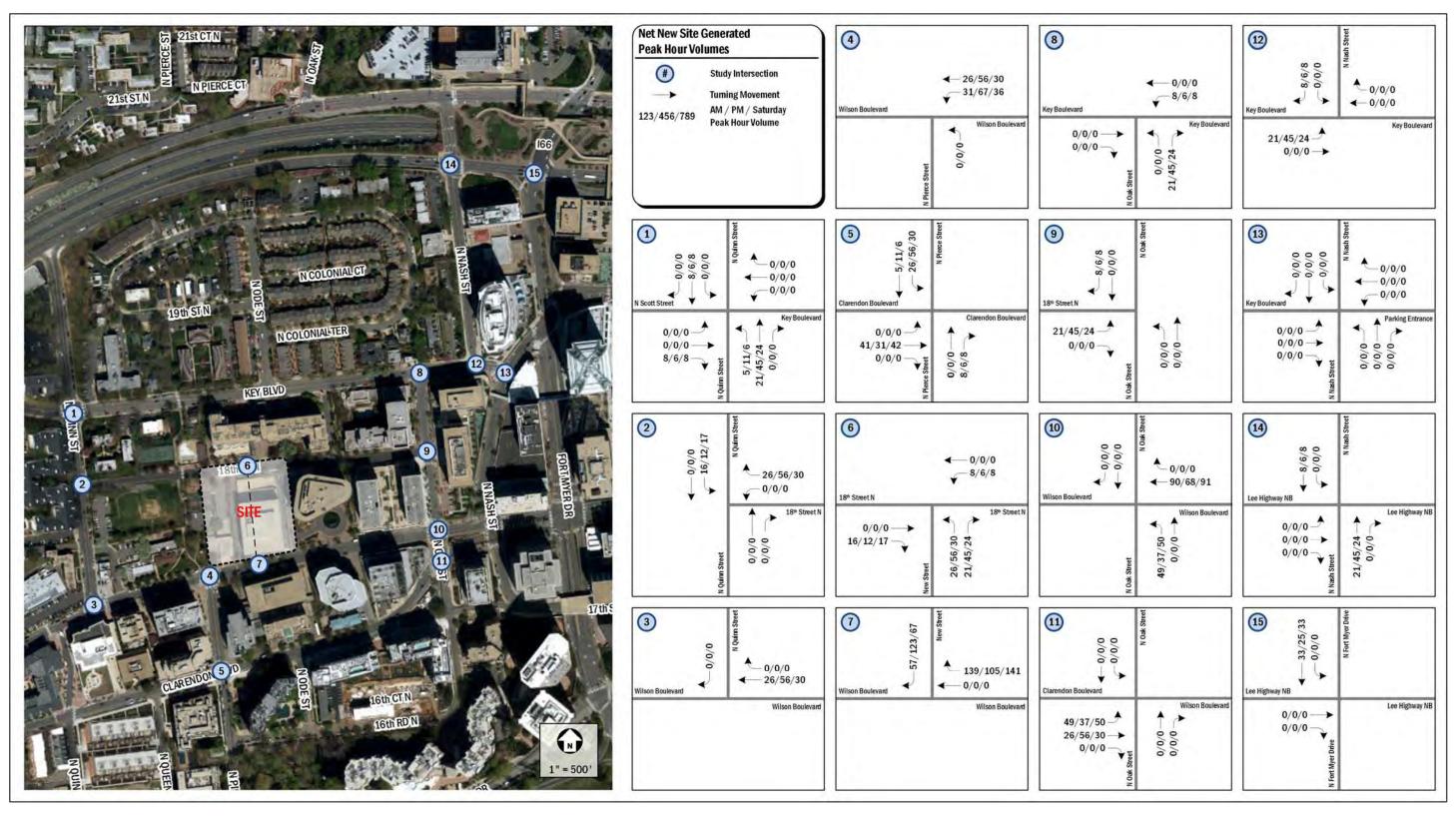


Figure 20: Net New Site Generated Peak Hour Volumes

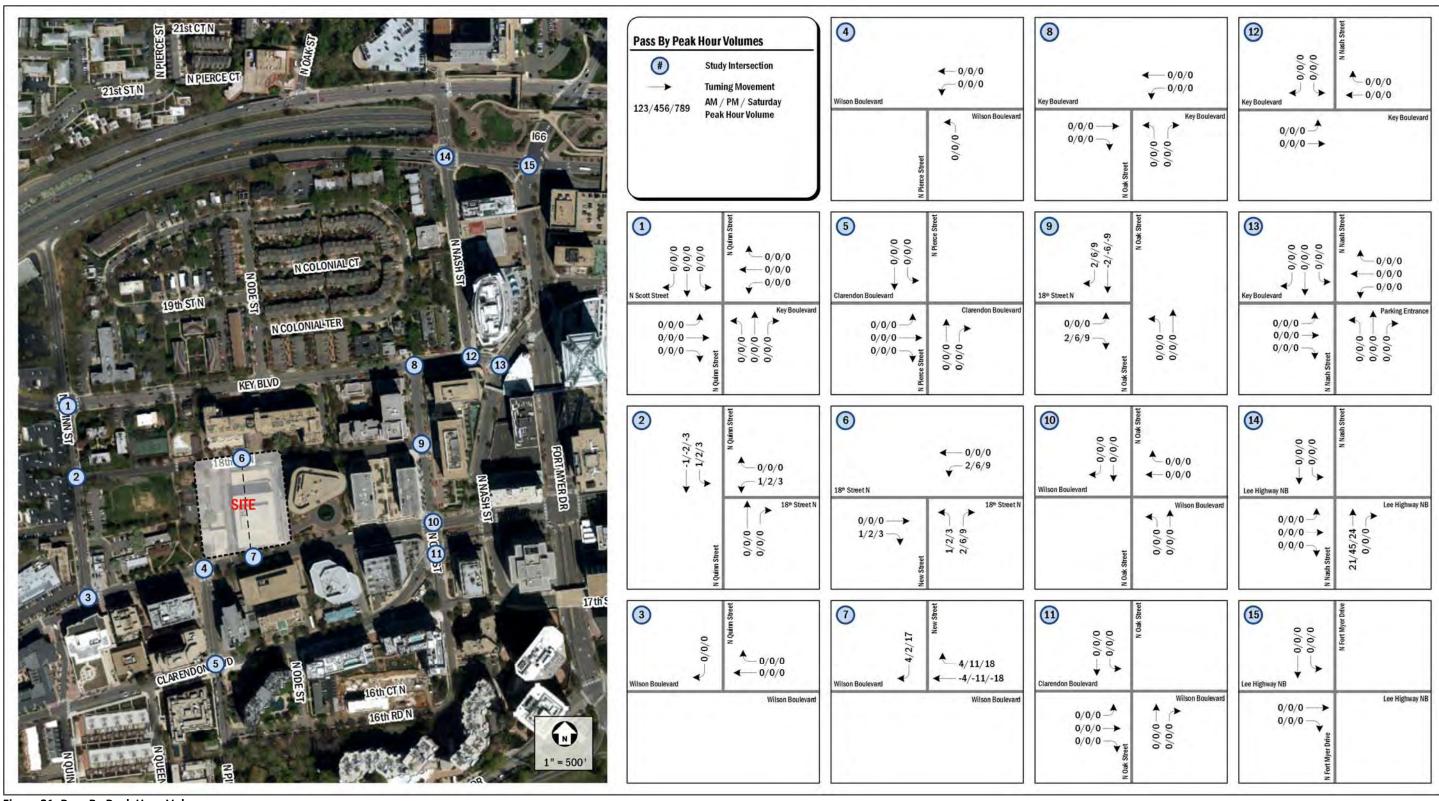


Figure 21: Pass-By Peak Hour Volumes

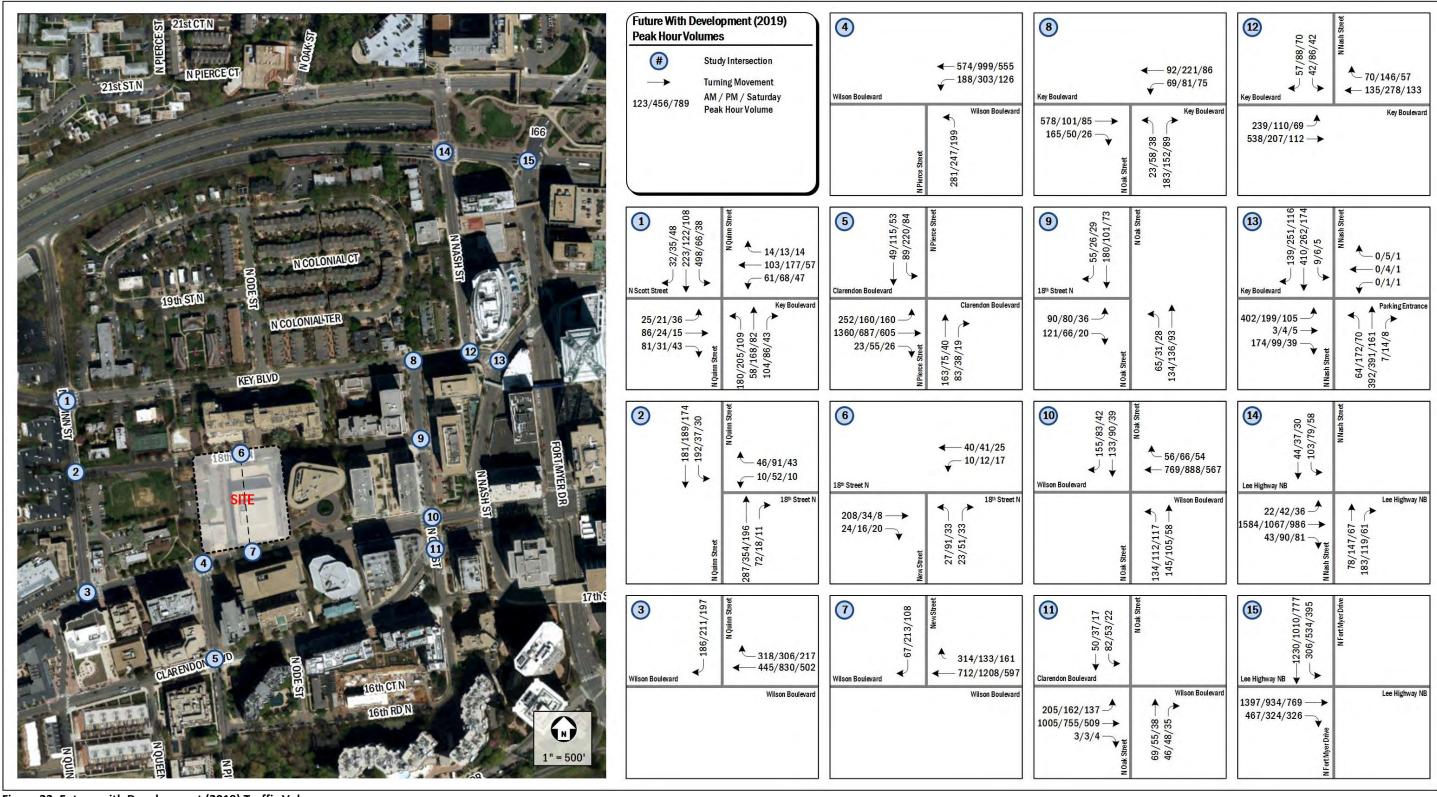


Figure 22: Future with Development (2019) Traffic Volumes

Future with Development Capacity Analysis

Capacity analyses were performed at the intersections within the study area during the weekday AM, weekday PM, and Saturday peak hours, for the future conditions with development scenario. *Synchro, Version 9.1* was used to analyze the study intersections based on the <u>Highway Capacity Manual</u> (HCM) methodology and include level of service, delay, and queue length comparisons for the turning movements analyzed. As previously described, a peak hour factor of 0.92 to 1.00 was used in the analysis of the future traffic conditions, consistent with VDOT practice.

The results of the intersection capacity analysis are presented in Table 7 and are expressed in level of service (LOS) and delay (seconds per vehicle) per lane group as shown in Figure 22. The 95% and 50% queue results for each intersection are also presented in Table 7, and are expressed in feet. The detailed analysis worksheets are included in Appendix F.

For the purpose of this analysis, it is desirable to achieve a level of service (LOS) of D or better for each lane group at the intersections. All movements at the study intersections operate at acceptable levels of service consistent with the Future without Development scenario, with the exception of the following:

- N Quinn Street and Key Boulevard (AM peak hour)
 - Southbound left/thru lanes (AM peak hour)
- N Pierce Street and Clarendon Boulevard
 - Southbound left/thru lane (PM peak hour)
- N Oak Street and Wilson Boulevard (AM peak hour)
 - Northbound left/thru lane (PM and Saturday peak hour)
- N Oak Street and Clarendon Boulevard
 - Eastbound left lane (PM peak hour)

All movements caused to operate below acceptable LOS thresholds by the proposed development have been mitigated with signal timing adjustments. These results are shaded and included in Table 7. The intersection of N Nash Street (east) and Key Boulevard currently operates below acceptable LOS thresholds. Historical counts and a field visit indicate vehicles are getting off Lee Highway at Quinn Street and using Key Boulevard as a cut-through to Rosslyn. Analysis of a signalized intersection has been included in Table 7 and a signal warrant analysis is included in later in the report.

Table 7: Future with Development (2019) Capacity Results

						Fu	uture			nt (2019)				
	Intersection (Movement)	Storage		A	M Peak			PI	VI Peak			Satur	day Pea	ık
	intersection (Movement)	Length		S Delay	Qı	ieue		C Delev	Qι	ieue	100	Delevi	Qı	ueue
			LO	S Delay	50tl	n 95th	LUS	S Delay	50th	95th	LUS	Delay	50tl	h 95th
1	N Quinn Street and Key Boulevard													
	Overall Intersection (Signalized)		Ε	63.1			В	19.5			В	13.5		
	Eastbound Left/Thru/Right	500	С	31.7	80	149	В	15.8	16	44	С	26.9	25	65
	Westbound Left/Thru/Right	315	С	34.8	92	163	В	19.6	106	172	С	28.8	55	105
	Northbound Left/Thru	200	Ε	68.5	150	#311	С	22.8	143	275	Α	5.5	37	44
	Northbound Right	50	Α	1.9	0	6	Α	9.4	4	m29	Α	1.8	0	m2
	Southbound Left/Thru	525	F	87.9	~515	#735	В	19.5	76	132	Α	8.5	37	65
	Southbound Right	75	Α	7.6	0	10	В	15.0	0	16	Α	7.7	0	14
1	N Quinn Street and Key Boulevard (Miti	gated)												
	Overall Intersection (Signalized)	,	D	41.8			-	-	-	-	_	-	-	_
	Eastbound Left/Thru/Right	500	D	40.1	89	163	-	-	-	-	_	-	-	_
	Westbound Left/Thru/Right	315	D	53.3	98	#211	_	-	_	_	_	-	_	_
	Northbound Left/Thru	200	С	23.9	95	#266	_	-	_	_	_	-	_	_
	Northbound Right	50	A	1.3	0	4	_	_	_	_	_	_	_	_
	Southbound Left/Thru	525	D	52.9	~432	#696	_	_	_	_	_	_	_	_
	Southbound Right	75	A	5.7	0	8	l <u>-</u>	_	_	_	_	_	-	_
2	N Quinn Street and 18th Street N	,,		5.,	J									
_	Overall Intersection (Unsignalized)		Α	3.2			Α	3.4			Α	1.8		
	Westbound Left/Right	315	В	14.0		13	c	15.1		23	В	10.8		8
	Northbound Thru/Right	375	A	0.0		0	A	0.0		0	A	0.0		0
	Southbound Left/Thru	200	A	8.9		18	A	8.3		3	A	7.8		3
3	N Quinn Street and Wilson Boulevard	200	Α	0.5		10		0.3		3		7.0		
3	Overall Intersection (Signalized)		С	21.8			В	11 5			С	20.5		
	, ,	225			Ε0	01		11.5	101	101			110	125
	Westbound Thru	325	A	9.3	58 25	81	A	7.8	101	101	С	20.3	146	135
	Westbound Right	85	С	27.3	25	m47	Α	7.8	7	11	С	27.4	14	26
_	Southbound Right	375	D	42.3	15	m24	С	31.9	85	159	В	13.4	80	139
4	N Pierce Street and Wilson Boulevard		_				_				_			
	Overall Intersection (Signalized)		В	15.9			С	20.3			В	18.4		
	Westbound Left/Thru	675	Α	6.2	91	m108	В	16.8	341	452	В	12.6	156	220
	Northbound Left	250	D	43.0	159	m217	D	37.4	129	206	D	38.4	119	185
5	N Pierce Street and Clarendon Boulevar	d												
	Overall Intersection (Signalized)		С	21.4			С	27.7			В	10.1		
	Eastbound Left/Thru	500	В	15.3	387	511	В	10.9	145	192	Α	5.5	84	146
	Eastbound Thru/Right	500	В	15.3	387	511	В	10.9	145	192	Α	5.5	84	146
	Northbound Thru/Right	185	D	36.0	120	201	С	25.6	42	87	С	32.6	21	52
	Southbound Left/Thru	250	Е	67.2	69	#191	Е	72.7	~228	#389	С	26.8	30	57
5	N Pierce Street and Clarendon Boulevard (Mitigated)												
	Overall Intersection (Signalized)		-	-	-	-	С	22.7			-	-	-	-
	Eastbound Left/Thru	500	-	-	-	-	В	12.1	155	205	-	-	-	-
	Eastbound Thru/Right	500	-	-	-	-	В	12.1	155	205	-	-	-	-
	Northbound Thru/Right	185	-	-	-	-	С	24.1	40	83	-	-	-	-
	Southbound Left/Thru	250	-	-	-	-	D	50.1	166	#371	-	-	-	-
6	New Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	1.8			Α	6.0			Α	5.3		
	Eastbound Thru/Right		Α	0.0		0	Α	0.0		0	Α	0.0		0
	Westbound Left/Thru		Α	7.8		0	Α	7.3		0	Α	7.3		0
	Northbound Left/Right		В	10.3		5	Α	9.7		13	Α	9.0		5
7	New Street and Wilson Boulevard	_			_	_			_					
	Overall Intersection (Unsignalized)		Α	0.9			Α	4.2			Α	1.6		
	Westbound Thru/Right		Α	0.0		0	Α	0.0		0	Α	0.0		0
	Southbound Right		В	14.0		13	D	30.3		140	В	12.6		18
8	N Oak Street and Key Boulevard						l							
_	Overall Intersection (Unsignalized)		С	20.1			Α	7.0			Α	5.1		
	Eastbound Thru/Right	285	A	0.0		0	A	0.0		0	Ā	0.0		0
	Westbound Left/Thru	150	В	11.8		10	A	8.4		5	A	7.8		5
	Northbound Left/Right	200	F	104.6		230	C	8.4 19.0		63	В	7.8 11.6		5 18
n		200	۲.	104.0		230	٦	13.0		US	-	11.0		10
9	N Oak Street and 18th Street N		_	6.3			_	4 7			_	2.0		
	Overall Intersection (Unsignalized)	050	Α	6.3		F-3	A	4.7		35	A	2.9		_
	Eastbound Left/Right	850	C	16.7		53	В	12.5		25	В	10.5		8
	Northbound Left/Thru	200	Α	8.2		5	Α	7.8		3	Α	7.7		3

						Fu	ıture	With De	velopme	ent (2019)				
	1.1	Storage		ΙA	И Peak				₁ Peak			Saturo	day Pea	ık
	Intersection (Movement)	Length	10	S Delay	Qı	ueue	10	S Delay	Qı	ueue	100	Delay	Qı	ieue
						h 95th	LO			h 95th	103			n 95th
	Southbound Thru/Right	200	Α	0.0		0	Α	0.0		0	Α	0.0		0
10	N Oak Street and Wilson Boulevard		_				_							
	Overall Intersection (Signalized)	425	E	75.2	422	474	C	29.7	450	204	C	27.2	00	110
	Westbound Thru Westbound Thru/Right	125 75	C	21.0 21.0	133 133	171 171	C C	21.6 21.6	159 159	201 201	B B	18.6 18.6	89 89	118 118
	Northbound Left/Thru	75 25	F	268.4	~247	m#296	E	64.5	143	m#207	E	57.3	115	m178
	Southbound Thru/Right	200	D	42.5	136	m200	C	30.6	67	132	C	26.1	19	57
10	N Oak Street and Wilson Boulevard (Mi			12.3	130	111200	Ü	30.0		132		20.1		<u> </u>
	Overall Intersection (Signalized)		С	31.9			С	28.4			С	25.0		
	Westbound Thru	125	С	27.5	153	197	С	24.0	172	218	В	17.2	85	113
	Westbound Thru/Right	75	С	27.5	153	197	С	24.0	172	218	В	17.2	85	113
	Northbound Left/Thru	25	D	52.2	181	m240	D	52.6	140	217	D	53.3	114	m177
	Southbound Thru/Right	200	С	24.9	102	m163	С	22.4	52	103	С	21.7	18	53
11	N Oak Street and Clarendon Boulevard													
	Overall Intersection (Signalized)		В	19.8	4		В	17.2	4.5-		В	17.9	e	
	Eastbound Left	75	F	88.2	123	m#191	E	58.7	102	m#167	D	54.9	85	#165
	Eastbound Thru/Right	550	A	5.9	67 46	99	A	7.8	77 20	m120	A	7.1	51	67 54
	Northbound Thru/Right Southbound Left/Thru	825 25	C B	29.2 10.8	46 14	94 m21	C A	28.0 9.4	30 12	73 15	C B	27.1 11.9	18 10	54 13
11	N Oak Street and Clarendon Boulevard		В	10.8	14	IIIZI	Α	5.4	12	12	В	11.9	10	13
11	Overall Intersection (Signalized)	(iviitigated)	В	16.5	_	_	В	14.9			_	_	_	_
	Eastbound Left	75	D	50.7	112	m#174	D	41.0	96	m144	_	_	_	_
	Eastbound Thru/Right	550	В	10.6	142	122	A	9.1	92	m137	_	_	_	_
	Northbound Thru/Right	825	В	18.8	41	80	C	23.0	25	64	_	_	-	-
	Southbound Left/Thru	25	Α	6.1	14	17	Α	7.7	12	15	-	-	-	-
12	N Nash Street and Key Boulevard													
	Overall Intersection (Unsignalized)		Α	5.0			Α	6.0			Α	3.9		
	Eastbound Left/Thru	135	Α	8.7		20	Α	9.1		10	Α	8.0		5
	Westbound Thru/Right	65	Α	0.0		0	Α	0.0		0	Α	0.0		0
	Southbound Left/Right	650	D	33.3		58	D	25.5		73	В	11.9		18
13	N Nash Street (east) and Key Boulevard													
	Overall Intersection (Unsignalized)	65	F	189.0			F	61.3		400	Α	4.2		20
	Eastbound Left	65 65	F D	738.5 25.9		933 75	F C	414.1 17.0		408	C B	16.6 11.2		28 5
	Eastbound Thru/Right Westbound Left/Thru/Right	25	A	0.0		0	D	29.0		28 5	В	13.3		0
	Northbound Left/Thru	400	A	9.5		8	A	9.2		18	A	8.0		5
	Northbound Thru/Right	400	A	0.4		0	A	0.6		0	A	0.1		0
	Southbound Left/Thru	150	Α	9.0		0	Α	9.0		0	Α	7.8		0
	Southbound Right	150	Α	0.0		0	Α	0.0		0	Α	0.0		0
13	N Nash Street (east) and Key Boulevard													
	Overall Intersection (Signalized)		С	20.3			В	13.8			В	12.2		
	Eastbound Left	65	С	34.8	230	m251	D	42.2	117	166	D	48.2	62	111
	Eastbound Thru/Right	65	С	21.5	36	m67	С	33.9	4	39	D	39.9	3	32
	Westbound Left/Thru/Right	25	Α	0.0	0	0	С	27.8	2	14	С	34.4	1	8
	Northbound Left/Thru	400	В	14.0	87	152	Α	7.0	70	136	Α	2.7	16	35
	Northbound Thru/Right	400	В	14.0	87	152	Α	7.0	70	136	Α	2.7	16	35
	Southbound Left/Thru	150	В	15.6	161	295	A	5.8	54	116	A	2.8	24	55 45
	Southbound Right N Nash Street and Lee Highway	150	В	12.3	0	36	Α	6.6	0	31	Α	2.7	0	15
14	Overall Intersection (Signalized)		В	17.4			В	16.3			В	10.2		
	Eastbound Left/Thru	1450	A	9.2	186	250	A	7.3	107	155	A	4.0	64	104
	Eastbound Thru	1450	Α	9.2	186	250	A	7.3	107	155	A	4.0	64	104
	Eastbound Thru/Right	1450	Α	9.2	186	250	Α	7.3	107	155	Α	4.0	64	104
	Northbound Thru/Right	650	D	48.2	146	m188	D	42.3	128	205	D	43.1	50	104
	Southbound Left	125	Ε	65.4	60	#140	Ε	56.0	46	#107	D	40.1	34	71
	Southbound Thru	125	С	28.9	22	49	С	29.5	19	42	D	36.3	17	42
15	N Fort Myer Drive and Lee Highway													
	Overall Intersection (Signalized)		С	20.6			В	17.6			В	18.4		
	Eastbound Thru	225	Α	9.2	114	122	В	17.3	103	123	Α	8.8	54	67
	Eastbound Right	70	В	11.6	96	130	С	20.7	86	126	В	10.5	55	81
	Southbound Left	250	С	32.9	143	251	В	18.9	160	265	С	26.9	83	177

					Fu	uture	With De	velopme	nt (2019)				
Intersection (Movement)	Storage		ΑI	VI Peak			PI	VI Peak			Satur	day Pea	k
intersection (iviovement)	Length	100	Delav	Qu	ieue	100	S Delav	Qu	ieue	100	Delav	Qu	eue
		LUS	Delay	50th	95th	LOS	belay	50th	95th	LUS	Delay	50th	95th
Southbound Left/Thru	250	С	34.0	268	331	В	16.5	178	221	С	26.9	164	210

m - Volume for 95th percentile queue is metered by upstream signal

^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

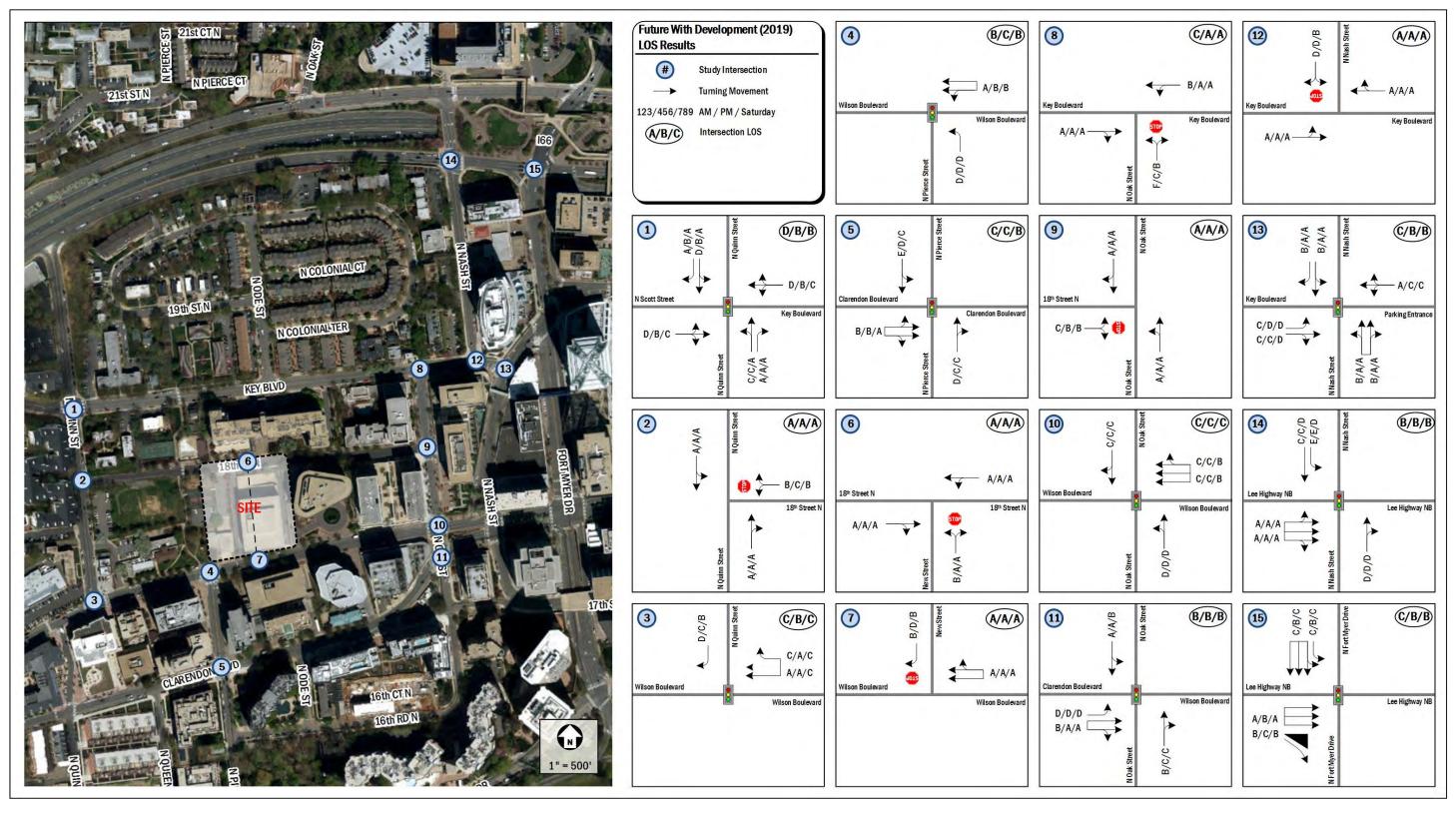


Figure 23: Future with Development (2019) LOS Results

FUTURE CONDITIONS WITH DEVELOPMENT (2025)

As requested by VDOT, a future with development scenario for 2025 has been analyzed. A regional growth rate of 0.5% annually has been applied for 2025 analysis scenario. Additional traffic volumes added to the network from the background growth are presented in Figure 24. Increase in traffic volumes for the full buildout of the background development projects identified is shown in Figure 25. Only the Rosslyn Plaza background development will see an increase in site trips from 2019 to 2025.

Future with Development Traffic Volumes

With no definitive changes to the roadway network between 2019 and 2025, there are no considerations for a rerouting of site trips. In order to determine the traffic volumes on the roadways in the vicinity of the development under the 2025 build condition, the future with development (2019) roadway volumes were added to the projected background growth and background development trips. The traffic volumes for the total future with development conditions in 2025 are presented in Figure 26.

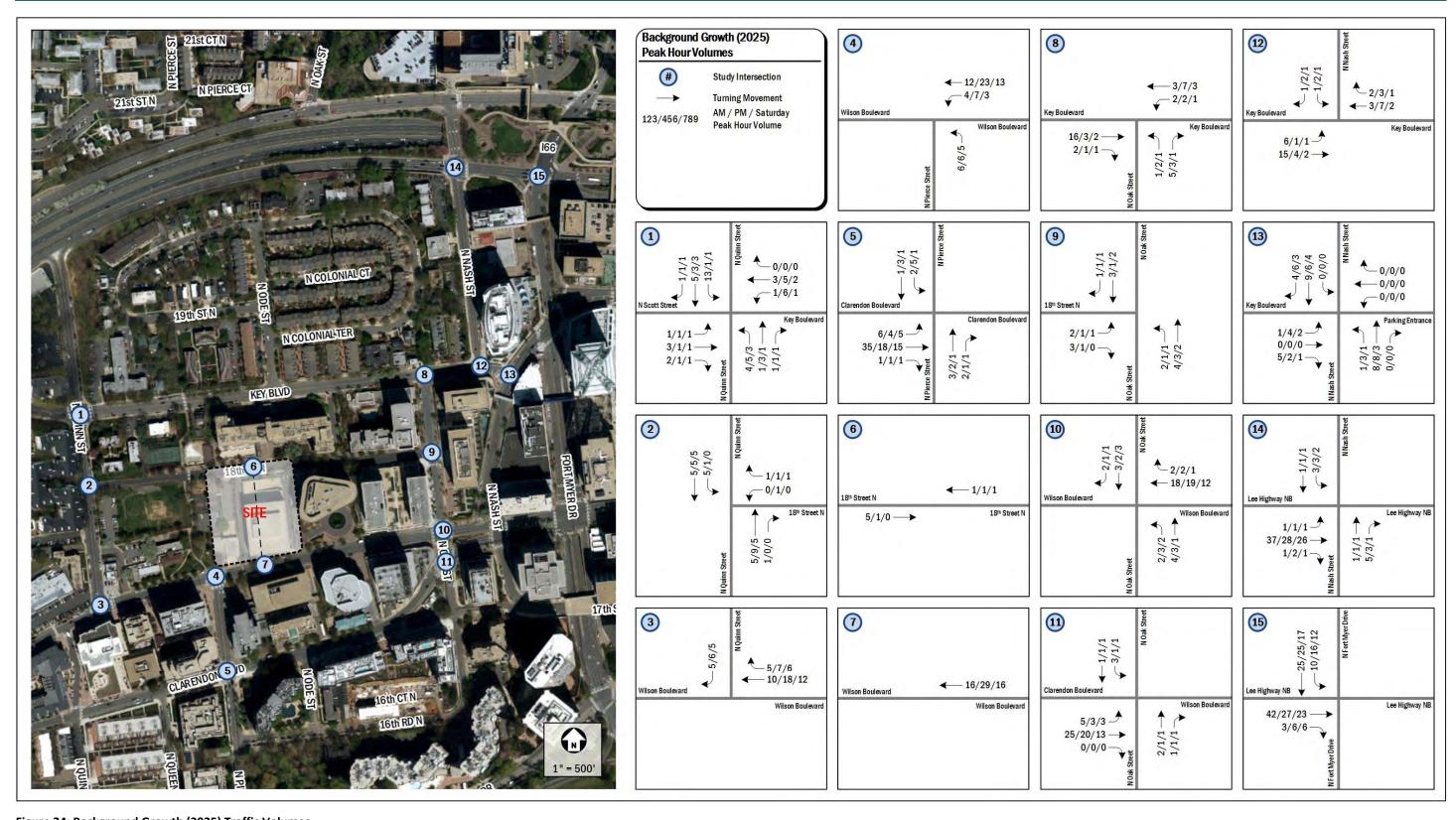


Figure 24: Background Growth (2025) Traffic Volumes

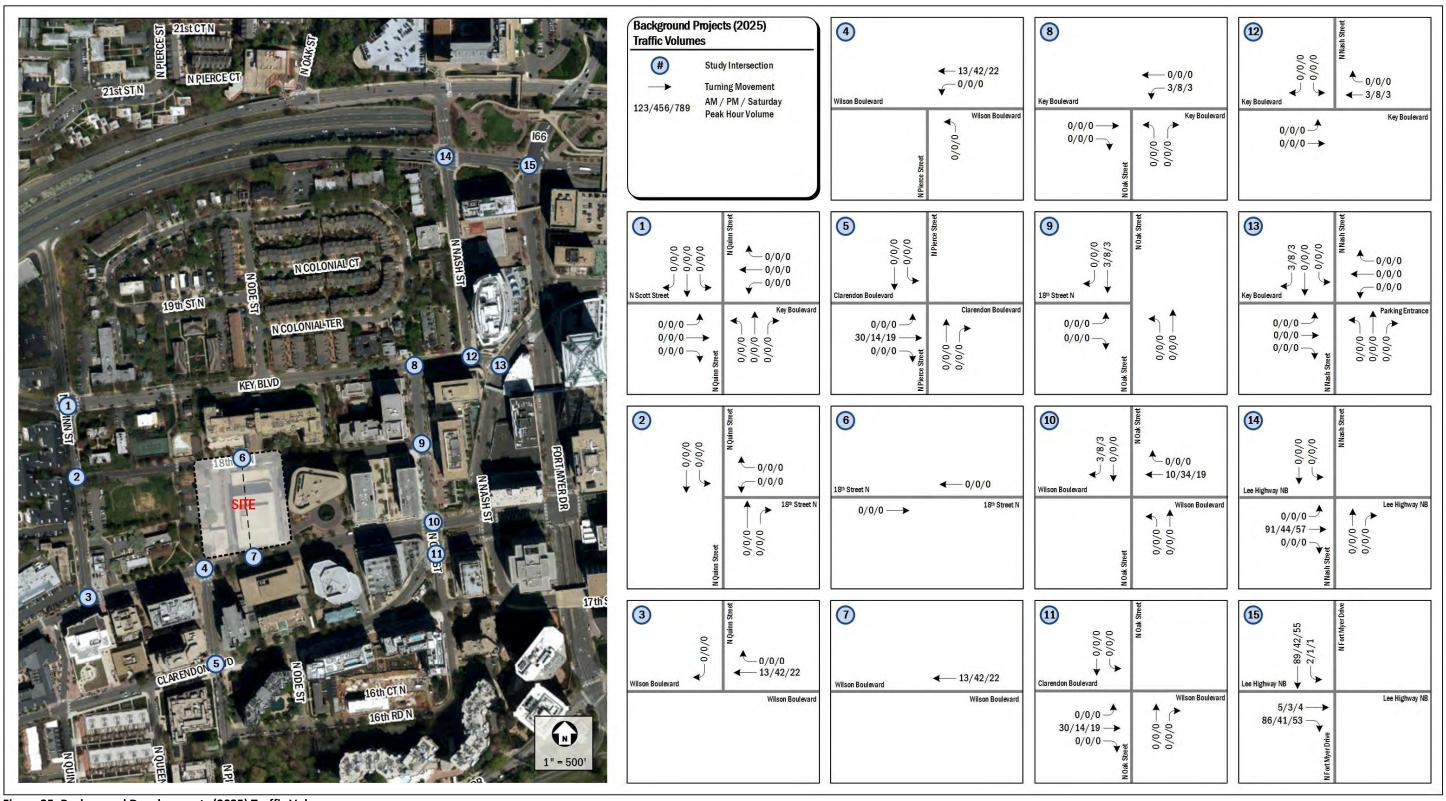


Figure 25: Background Developments (2025) Traffic Volumes

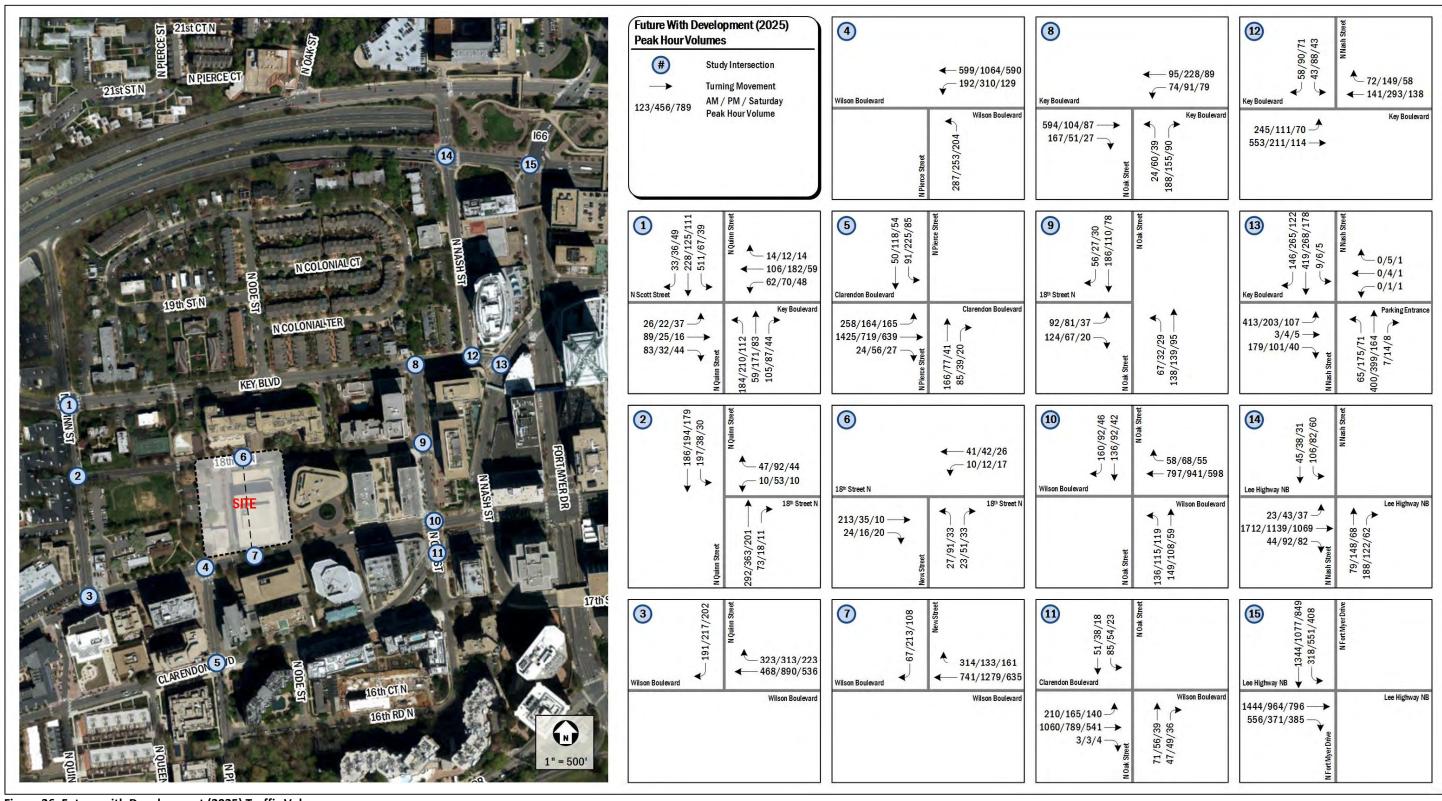


Figure 26: Future with Development (2025) Traffic Volumes

Future with Development Capacity Analysis

Capacity analyses were performed at the intersections within the study area during the weekday AM, weekday PM, and Saturday peak hours, for the future conditions with development scenario. *Synchro, Version 9.1* was used to analyze the study intersections based on the <u>Highway Capacity Manual</u> (HCM) methodology and include level of service, delay, and queue length comparisons for the turning movements analyzed. As previously described, a peak hour factor of 0.92 to 1.00 was used in the analysis of the future traffic conditions, consistent with VDOT practice.

The results of the intersection capacity analysis are presented in Table 8 and are expressed in level of service (LOS) and delay (seconds per vehicle) per lane group as shown in Figure 27. The 95% and 50% queue results for each intersection are also presented in Table 8 and are expressed in feet. The detailed analysis worksheets are included in Appendix F.

Under future with development (2025) conditions, all intersection movements within the study area continue to operate consistent with future with development (2019) results with the exception of the following:

- N Quinn Street and Key Boulevard
 - Westbound left/thru/right lane (AM peak hour)
 - Southbound left/thru lane (AM peak hour)

Other movements caused to operate below acceptable LOS thresholds by the proposed development in 2019 have been mitigated with signal timing adjustments. These results are shaded and included in Table 8. Although movements at the intersection of N Quinn Street and Key Boulevard exceeds County standards, the 2025 analysis was done as a planning scenario; therefore, no mitigation is proposed at this intersection.

Table 8: Future with Development (2025) Capacity Results

						Fu	ture \			ent (2025)				
	Intersection (Movement)	Storage Length	ļ	ΑI	M Peak			PΝ	/I Peak			Satur	day Pea	ak
	intersection (wiovement)	Storage Length	ıo	S Delay		ueue	109	Delay		ueue	109	S Delay		ueue
				3 Delay	50t	h 95th		Delay	50t	h 95th	í	Delay	50tl	h 95th
1	N Quinn Street and Key Boulevard													
	Overall Intersection (Signalized)		D	47.7			С	21.0			В	13.5		
	Eastbound Left/Thru/Right	500	D	41.1	93	171	В	15.8	17	45	С	27.0	26	68
	Westbound Left/Thru/Right	315	Е	56.2	99	#221	С	23.2	118	210	С	28.4	54	97
	Northbound Left/Thru	200	С	30.1	98	#284	С	24.1	144	#294	Α	5.5	38	45
	Northbound Right	50	Α	1.3	0	4	В	10.4	4	m32	Α	1.8	0	m2
	Southbound Left/Thru	525	Е	61.6	~502	#723	В	19.7	78	136	Α	8.6	38	67
	Southbound Right	75	Α	5.7	0	8	В	15.0	0	17	Α	7.7	0	14
2	N Quinn Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	3.2			Α	3.4			Α	1.7		
	Westbound Left/Right	315	В	14.2		13	В	15.4		33	В	10.9		8
	Northbound Thru/Right	375	Α	0.0		0	Α	0.0		0	Α	0.0		0
	Southbound Left/Thru	200	Α	9.0		18	Α	8.3		3	Α	7.9		3
3	N Quinn Street and Wilson Bouleva	ard												
-	Overall Intersection (Signalized)		С	21.5			В	11.3			С	20.1		
	Westbound Thru	325	A	9.2	58	84	A	7.6	104	105	c	20.3	149	14:
	Westbound Right	85	c	25.8	24	m45	A	6.6	6	m7	C	25.6	12	25
	Southbound Right	375	D	44.3	22	m34	c	33.3	94	168	В	13.5	82	14
4	N Pierce Street and Wilson Bouleva		Ť	5			Ť	55.5	٠,	-50	_		J.	
•	Overall Intersection (Signalized)		В	15.9			С	22.2			В	18.5		
	Westbound Left/Thru	675	A	6.3	95	m111	В	19.2	406	497	В	12.8	165	23:
	Northbound Left	250	D	43.1	156	m217	D	37.8	131	209	D	38.6	122	190
5	N Pierce Street and Clarendon Bou			73.1	130	111217		37.0	131	203		30.0	122	150
3	Overall Intersection (Signalized)	ievaiu	С	20.4			С	24.0			В	10.4		
	Eastbound Left/Thru	500	В	15.1	351	#705	В	12.5	165	217	A	5.7	90	15
	Eastbound Thru/Right	500	В	15.1	351	#705 #705	В	12.5	165	217	A	5.7 5.7	90	15
		185	С	34.1	131	185	C	24.1	42	86	C	32.6	22	54
	Northbound Thru/Right	250	E	60.5	45	#165	D	55.0	160	m#369	C	28.4	30	60
_	Southbound Left/Thru New Street and 18 th Street N	250		60.5	45	#105	D	55.0	160	111#309	C	28.4	30	60
6				4.0				- 0				F 2		
	Overall Intersection (Unsignalized)		A	1.8		0	A	5.9		•	A	5.2		•
	Eastbound Thru/Right		Α	0.0		0	A	0.0		0	A	0.0		0
	Westbound Left/Thru		Α	7.8		0	A	7.3		0	A	7.3		0
	Northbound Left/Right		В	10.4		5	Α	9.7		15	Α	9.0		5
7	New Street and Wilson Boulevard		١.	• •			_							
	Overall Intersection (Unsignalized)		A	0.8		•	A	4.5		•	A	1.5		•
	Westbound Thru/Right		Α	0.0		0	Α	0.0		0	Α	0.0		0
	Southbound Right		В	14.2		15	D	34.3		115	В	12.9		20
8	N Oak Street and Key Boulevard													
	Overall Intersection (Unsignalized)		С	23.8			Α	7.4			Α	5.2		
	Eastbound Thru/Right	285	Α	0.0		0	Α	0.0		0	Α	0.0		0
	Westbound Left/Thru	150	В	12.0		13	Α	8.4		8	Α	7.8		5
	Northbound Left/Right	200	F	123.9		253	С	20.1		68	В	11.7		20
9	N Oak Street and 18th Street N													
	Overall Intersection (Unsignalized)		Α	6.4			Α	4.7			Α	2.9		
	Eastbound Left/Right	850	С	17.2		58	В	12.8		25	В	10.6		8
	Northbound Left/Thru	200	Α	8.2		5	Α	7.9		3	Α	7.7		3
	Southbound Thru/Right	200	Α	0.0		0	Α	0.0		0	Α	0.0		0
10	N Oak Street and Wilson Boulevard	d												
	Overall Intersection (Signalized)		С	32.5			С	29.0			С	25.0		
	Westbound Thru	125	С	27.9	160	205	С	24.7	185	234	В	17.4	90	120
	Westbound Thru/Right	75	С	27.9	160	205	С	24.7	185	234	В	17.4	90	120
	Northbound Left/Thru	25	D	54.2	186	m240	D	54.0	144	224	D	53.5	116	m17
	Southbound Thru/Right	200	С	25.1	107	m169	С	22.8	55	108	С	21.9	19	56
11	N Oak Street and Clarendon Bouler													
	Overall Intersection (Signalized)	-	В	16.8			В	15.2			В	18.0		
	Eastbound Left	75	D	51.9	116	m#157	D	41.0	98	m148	D	47.5	87	15
	Eastbound Thru/Right	550	В	11.0	144	127	A	9.7	102	m149	A	7.7	56	76
	Northbound Thru/Right	825	В	18.9	43	83	c	23.0	26	66	c	26.4	18	54
	Southbound Left/Thru	25	A	6.1	43 14	17	A	7.8	13	16	В	12.8	11	15
	N Nash Street and Key Boulevard	23	^	0.1	14	1/	Α.	7.0	13	10	В	12.0	11	13

						Fu	ture \	Nith De	velopm	ent (2025))			
	Intersection (Movement)	Storage Length		ΙA	VI Peak			PN	/I Peak			Satur	day Pea	k
	intersection (Movement)	Storage Length	LOS	S Delay		ueue h 95th	LOS	Delay		ueue h 95th	LOS	Delay		ieue n 95th
	Overall Intersection (Unsignalized)	Α	5.3			Α	6.4			Α	3.9		
	Eastbound Left/Thru	135	Α	8.8		20	Α	9.2		10	Α	8.0		5
	Westbound Thru/Right	65	Α	0.0		0	Α	0.0		0	Α	0.0		0
-	Southbound Left/Right	650	Ε	37.4		65	D	27.4		80	В	12.0		18
13	N Nash Street (east) and Key Boul	evard												
	Overall Intersection (Signalized)		С	20.8			В	13.9			В	11.4		
	Eastbound Left	65	D	35.5	234	m261	D	42.3	120	168	D	42.3	63	112
	Eastbound Thru/Right	65	С	21.7	40	m71	С	33.2	4	39	D	37.9	3	32
	Westbound Left/Thru/Right	25	Α	0.0	0	0	С	27.6	2	14	С	33.0	1	8
	Northbound Left/Thru	400	В	14.5	93	153	Α	7.2	73	142	Α	3.2	16	36
	Northbound Thru/Right	400	В	14.5	93	153	Α	7.2	73	142	Α	3.2	16	36
	Southbound Left/Thru	150	В	16.2	170	296	Α	5.9	56	120	Α	3.2	25	56
	Southbound Right	150	В	12.7	0	37	Α	6.9	0	32	Α	3.1	0	16
14	N Nash Street and Lee Highway													
	Overall Intersection (Signalized)		В	17.8			В	17.3			В	10.5		
	Eastbound Left/Thru	1450	В	10.0	215	282	Α	7.6	117	168	Α	4.1	71	115
	Eastbound Thru	1450	В	10.0	215	282	Α	7.6	117	168	Α	4.1	71	115
	Eastbound Thru/Right	1450	В	10.0	215	282	Α	7.6	117	168	Α	4.1	71	115
	Northbound Thru/Right	650	D	48.4	142	m201	D	47.1	132	222	D	48.0	51	111
	Southbound Left	125	Ε	67.8	62	#147	E	60.9	48	#114	D	40.5	35	72
	Southbound Thru	125	С	28.6	22	50	С	29.3	19	43	D	36.3	18	43
15	N Fort Myer Drive and Lee Highwa	ıy												
	Overall Intersection (Signalized)		С	23.5			В	18.3			В	19.1		
	Eastbound Thru	225	Α	9.2	116	125	В	17.3	105	126	Α	8.8	55	68
	Eastbound Right	70	В	14.7	115	150	С	23.9	100	141	В	11.6	65	92
	Southbound Left	250	С	34.1	151	265	В	19.8	175	290	С	28.4	104	208
	Southbound Left/Thru	250	D	40.0	304	#407	В	16.9	191	237	С	27.6	180	229

m - Volume for 95th percentile queue is metered by upstream signal

^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

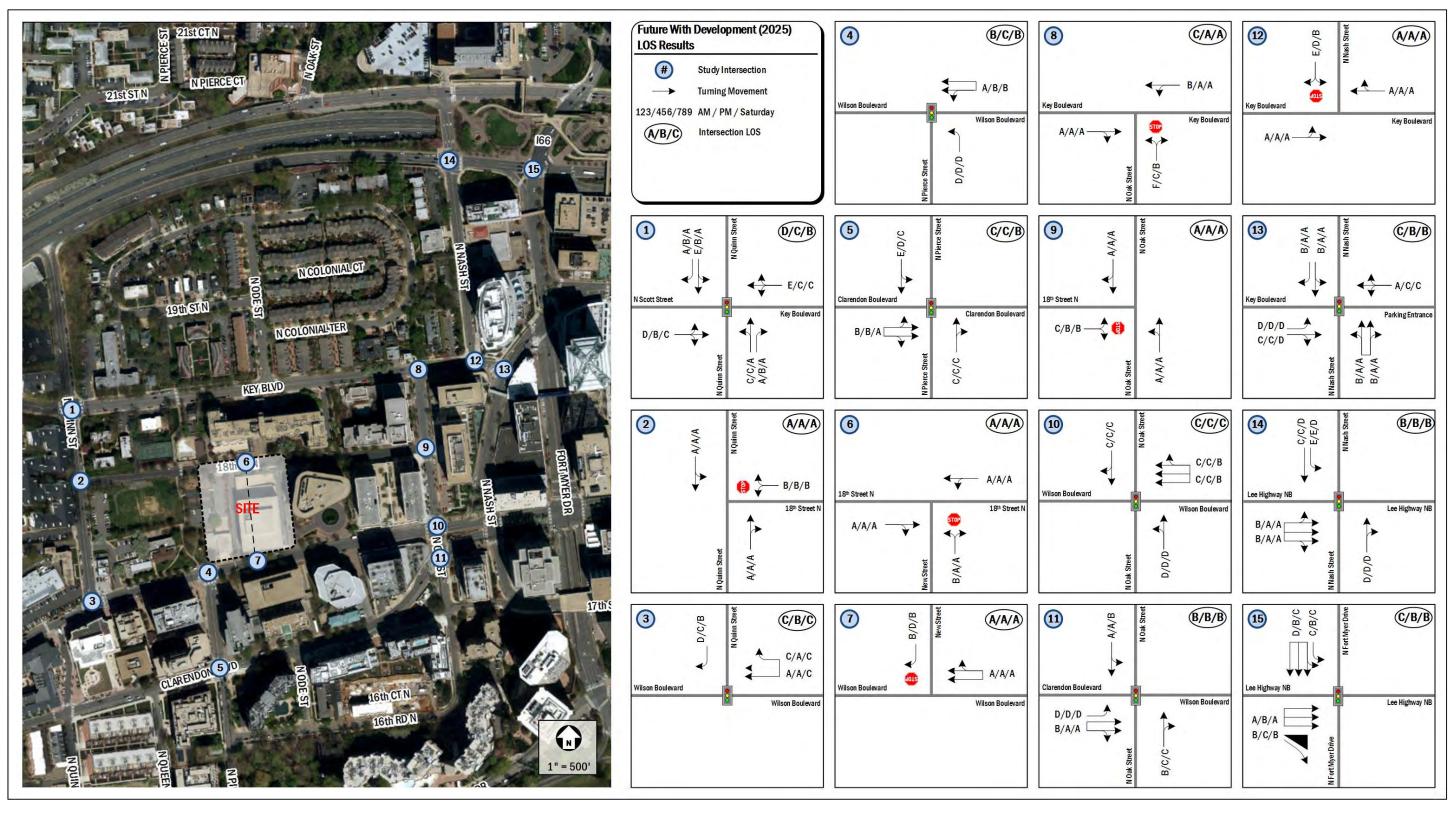


Figure 27: Future with Development (2025) LOS Results

SUMMARY OF VEHICULAR CAPACITY ANALYSIS

As described in the previous sections, vehicular capacity analysis was performed for the following three scenarios:

- Existing (2016) Conditions based on existing turning movement counts.
- Future without Development (2019) Conditions –assumes approved developments in the vicinity of the subject project and growth on the road network.
- **Future with Development (2019) Conditions** assumes approved developments in the vicinity of the subject project with growth and new traffic generated by the subject project.
- Future with Development (2025) Conditions assumes approved developments in the vicinity of the subject project with growth and new traffic generated by the subject project.

A comparison of the LOS and delay results for the four scenarios is presented in Table 9. No mitigation beyond adjustment of signal timing is proposed for the study area intersections. All movements caused to operate below acceptable LOS thresholds by the proposed development have been mitigated with signal timing adjustments. These results are shaded in the table below.

Table 9: AM Peak Hour Comparison of Capacity Analysis Results

			Existing	2016	D	Future V evelopme	vithout ent (2019)	D	Future 'evelopme		Dev	Future V	With nt (2025)
Intersection (Movement)			AM P	eak		AM P	eak		AM P	eak		AM Pe	eak
, ,		LO	S Delay	95th Queue	LOS	S Delay	95th Queue	LO	S Delay	95th Queue	LOS	Delay	95th Queue
1) N Quinn Street and Key Boul	evard				1								
Overall Intersection (Signalized)	С	32.4		D	54.1		E	63.1		-	-	
Eastbound Left/Thru/Right	500	С	31	132	С	31.3	144	С	31.7	149	-	-	-
Westbound Left/Thru/Right	315	С	32.9	143	С	34.6	162	С	34.8	163	-	-	-
Northbound Left/Thru	200	В	17.6	98	D	49.6	#274	Е	68.5	#311	-	-	-
Northbound Right	50	Α	2.3	4	Α	1.8	5	Α	1.9	6	-	-	-
Southbound Left/Thru	525	D	39.1	#550	Е	75.9	717	F	87.9	#735	-	-	-
Southbound Right	75	Α	7.6	7	Α	7.6	10	Α	7.6	10	-	-	-
1) N Quinn Street and Key Boul	evard (Miti	gated)										
Overall Intersection (Signalized)	-	-		-	-		D	41.8		D	47.7	
Eastbound Left/Thru/Right	500	-	-	-	-	-	-	D	40.1	163	D	41.1	171
Westbound Left/Thru/Right	315	-	-	-	-	-	-	D	53.3	#211	Ε	56.2	#221
Northbound Left/Thru	200	-	-	-	-	-	-	С	23.9	#266	С	30.1	#284
Northbound Right	50	-	-	-	-	-	-	Α	1.3	4	Α	1.3	4
Southbound Left/Thru	525	-	-	-	-	-	-	D	52.9	#696	Е	61.6	#723
Southbound Right	75	-	-	-	-	-	-	Α	5.7	8	Α	5.7	8
2) N Quinn Street and 18th Stre	eet N												
Overall Intersection (Unsignalize	zed)	Α	2.8		Α	2.6		Α	3.2		Α	3.2	
Westbound Left/Right	315	В	12.3	5	В	14.8	8	В	14	13	В	14.2	13
Northbound Thru/Right	375	Α	0	0	Α	0	0	Α	0	0	Α	0	0
Southbound Left/Thru	200	Α	8.2	13	Α	8.8	15	Α	8.9	18	Α	9	18
3) N Quinn Street and Wilson B					1								
Overall Intersection (Signalized	í	С	34.4		С	23.4		С	21.8		С	21.5	
Westbound Thru	325	В	10	61	Α	9.5	79	Α	9.3	81	Α	9.2	84
Westbound Right	85	С	23.8	28	С	29.4	m50	С	27.3	m47	С	25.8	m45
Southbound Right	375	F	93.8	m15	D	44.4	m22	D	42.3	m24	D	44.3	m34
4) N Pierce Street and Wilson B		۱.	12.5		١.	16.1		_	15.0		_	45.0	
Overall Intersection (Signalized	IJ	В	12.5		В	16.1		В	15.9		В	15.9	

			Existing	2016		Future V			Future \			Future \	
					D	evelopme		D	evelopme AM Pe		Dev	•	nt (2025)
Intersection (Movement)			AM F			AM P			AIVI PE			AM Pe	
		LO	S Delay	95th Queue	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LOS	Delay	95th Queue
Westbound Left/Thru	675	Α	3.9	58	Α	6.1	m97	Α	6.2	m108	Α	6.3	m111
Northbound Left	250	D	37.7	162	D	41.8	m#228	D	43	m217	D	43.1	m217
5) N Pierce Street and Clarendo		1			ı _			_			_		
Overall Intersection (Signalized Eastbound Left/Thru	I) 500	B A	13.6 8.1	339	В В	18.2 12	482	C B	21.4 15.3	511	C B	20.4 15.1	#705
Eastbound Thru/Right	500	A	8.1	339	В	12	482	В	15.3	511	В	15.1	#705 #705
Northbound Thru/Right	185	D	39.5	147	D	41.9	195	D	36	201	С	34.1	185
Southbound Left/Thru	250	D	41.1	64	Е	55.6	#142	Е	67.2	#191	Е	60.5	#165
6) New Street and 18 th Street N		1			ı								
Overall Intersection (Unsignalize	zed)	-	-		-	-		Α	1.8		Α	1.8	
Eastbound Thru/Right		-	-	-	-	-	-	Α	0	0	Α	0	0
Westbound Left/Thru		-	-	-	-	-	-	Α	7.8	0	Α	7.8	0
Northbound Left/Right		-	-	-	-	-	-	В	10.3	5	В	10.4	5
7) New Street and Wilson Boul	evard												
Overall Intersection (Unsignalize	zed)	-	_		-	-		Α	0.9		Α	0.8	
Westbound Thru/Right		-	-	-	-	-	_	Α	0	0	Α	0	0
Southbound Right		-	-	-	-	-	-	В	14	13	В	14.2	15
8) N Oak Street and Key Boulev	ard .												
Overall Intersection (Unsignalize	zed)	В	13.6		В	14.3		С	20.1		С	23.8	
Eastbound Thru/Right	285	Α	0	0	Α	0	0	Α	0	0	Α	0	0
Westbound Left/Thru	150 200	B	10.8 65.1	8 165	B	11.7 79.8	10 183	B	11.8 104.6	10 230	B F	12 123.9	13
Northbound Left/Right 9) N Oak Street and 18th Street		F	05.1	103	Г	79.0	103	Г	104.0	<mark>250</mark>	Г	123.9	253
Overall Intersection (Unsignalize		Α	5.7		Α	5.6		Α	6.3		Α	6.4	
Eastbound Left/Right	850	В	13.1	33	c	15.3	43	c	16.7	53	c	17.2	58
Northbound Left/Thru	200	Α	7.8	8	Α	8.1	5	Α	8.2	5	A	8.2	5
Southbound Thru/Right	200	Α	0	0	Α	0	0	Α	0	0	Α	0	0
10) N Oak Street and Wilson Bo		1 _			1_			_					
Overall Intersection (Signalized	ĺ	С	30.2		D	46		E	75.2		-	-	
Westbound Thru	125	В	19.7	130	В	20	150	С	21	171	-	-	-
Westbound Thru/Right	75	В	19.7	130	В	20	150	С	21	171	-	-	-
Northbound Left/Thru	25	Е	58.4	m#228	F	133.2	m#286	F	268.4	m#296	-	-	-
Southbound Thru/Right	200	С	34.9	m144	D	42.7	m200	D	42.5	m200	-	-	-
10) N Oak Street and Wilson Bo	oulevard (M	itigat	ed)										
Overall Intersection (Signalized	1)	-	-		-	-		С	31.9		С	32.5	
Westbound Thru	125	-	-	-	-	-	-	С	27.5	197	С	27.9	205
Westbound Thru/Right	75	-	-	-	-	-	-	С	27.5	197	С	27.9	205
Northbound Left/Thru	25	-	-	-	-	-	-	D	52.2	m240	D	54.2	m240
Southbound Thru/Right	200	-	-	-	_	-	-	С	24.9	m163	С	25.1	m169
11) N Oak Street and Clarendo	n Boulevard												
Overall Intersection (Signalized	1)	В	14.6		В	14.1		В	19.8		-	-	
Eastbound Left	75	D	52.7	m#171	Ε	56.5	m126	F	88.2	m#191	-	-	-
Eastbound Thru/Right	550	Α	6.2	64	Α	6	91	Α	5.9	99	-	-	-
Northbound Thru/Right	825	С	29.2	88	С	29.1	93	С	29.2	94	-	-	-
Southbound Left/Thru	25	Α	8.1	10	В	10.8	m21	В	10.8	m21	-	-	-
11) N Oak Street and Clarendon	n Boulevard	(Miti	gated)										
Overall Intersection (Signalized	1)	-	-		-	-		В	16.5	-	В	16.8	
Eastbound Left	75	-	-	-	-	-	-	D	50.7	m#174	D	51.9	m#157
Eastbound Thru/Right	550	-	-	-	-	-	-	В	10.6	122	В	11	127
	ı	1											

			Existing	2016	D	Future Wevelopme		De	Future \ evelopmer		Dev	Future '	With nt (2025)
Intersection (Movement)			AM P	eak		AM P	eak		AM Pe	ak		AM Pe	eak
		LO	S Delay	95th Queue	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LOS	6 Delay	95th Queue
Northbound Thru/Right	825	-	-	-	-	-	-	В	18.8	80	В	18.9	83
Southbound Left/Thru	25	-	-	-	-	-	-	Α	6.1	17	Α	6.1	17
12) N Nash Street and Key Bou	levard												
Overall Intersection (Unsignalis	zed)	Α	3.1		Α	4.5		Α	5		Α	5.3	
Eastbound Left/Thru	135	Α	8.5	15	Α	8.6	18	Α	8.7	20	Α	8.8	20
Westbound Thru/Right	65	Α	0	0	Α	0	0	Α	0	0	Α	0	0
Southbound Left/Right	650	С	19	20	D	30.7	48	D	33.3	58	Е	37.4	65
13) N Nash Street (east) and Ke	y Boulevar	d											
Overall Intersection (Unsignalia	zed)	F	102.1		F	189		F	189		-	-	
Eastbound Left	65	F	351.4	658	F	738.5	933	F	738.5	933	-	-	-
Eastbound Thru/Right	65	С	19.7	53	D	25.9	75	D	25.9	75	_	-	-
Westbound Left/Thru/Right	25	Α	0	0	Α	0	0	Α	0	0	_	_	_
Northbound Left/Thru	400	Α	9	5	Α	9.5	8	Α	9.5	8	_	_	_
Northbound Thru/Right	400	Α	0.2	0	Α	0.4	0	A	0.4	0	_	_	_
Southbound Left/Thru	150	A	8.7	0	Α	9	0	Α	9	0	_	_	_
Southbound Right	150	A	0	0	A	0	0	A	0	0	_	_	_
13) N Nash Street (east) and Ke								,,					
	•	۱۱۱۷۱۱) 	ilgateuj		I			С	20.2		С	20.0	
Overall Intersection (Signalized	ĺ	-	-		-	-			20.3	254		20.8	264
Eastbound Left	65	-	-	-	-	-	-	С	34.8	m251	D	35.5	m261
Eastbound Thru/Right	65	-	-	-	-	-	-	С	21.5	m67	С	21.7	m71
Westbound Left/Thru/Right	25	-	-	-	-	-	-	Α	0	0	Α	0	0
Northbound Left/Thru	400	-	-	-	-	-	-	В	14	152	В	14.5	153
Northbound Thru/Right	400	-	-	-	-	-	-	В	14	152	В	14.5	153
Southbound Left/Thru	150	-	-	-	-	-	-	В	15.6	295	В	16.2	296
Southbound Right	150	-	-	-	-	-	-	В	12.3	36	В	12.7	37
14) N Nash Street and Lee High	way												
Overall Intersection (Signalized	I)	В	16.2		В	16.2		В	17.4		В	17.8	
Eastbound Left/Thru	1450	Α	6.9	172	Α	8.9	250	Α	9.2	250	В	10	282
Eastbound Thru	1450	Α	6.9	172	Α	8.9	250	Α	9.2	250	В	10	282
Eastbound Thru/Right	1450	Α	6.9	172	Α	8.9	250	Α	9.2	250	В	10	282
Northbound Thru/Right	650	D	45.7	m134	D	46.9	m174	D	48.2	m188	D	48.4	m201
Southbound Left	125	E	64.9	111	E	56.6	#131	E	65.4	#140	E	67.8	#147
Southbound Thru	125	С	30.6	36	С	29.2	42	С	28.9	49	С	28.6	50
15) N Fort Myer Drive and Lee	• .	1	17 5		۱.	20.2		_	20.6			22 5	
Overall Intersection (Signalized Eastbound Thru	· ·	В В	17.5	126	C ^	20.2	125	C A	20.6	122	C	23.5	125
Eastbound Inru Eastbound Right	225 70	В	10.8 7.4	136 29	A B	9.4 11.8	125 130	B	9.2 11.6	122 130	A B	9.2 14.7	125 150
Southbound Left	250	C	7.4 31.9	239	C	32.9	251	C	32.9	251	C	34.1	265
Southbound Left/Thru	250	c	26.3	198	c	33	319	C	34	331	D	40	#407

m - Volume for 95th percentile queue is metered by upstream signal

^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

Table 10: PM Peak Hour Comparison of Capacity Analysis Results

			Existing	2016	ח	Future W evelopme		De	Future \ evelopme		Dev	Future elopme	
Intersection (Movement)	Storage	PM Peak				PM P	• •		PM Pe		Development (2025 PM Peak		
intersection (Movement)	(ft)	LOS	Delay	95th Queue	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LOS	Delay	95th Queue
1) N Quinn Street and Key Boul	evard												
Overall Intersection (Signalized)	В	15.5		В	16.9		В	19.5		-	-	
Eastbound Left/Thru/Right	500	В	15.6	35	В	15.7	43	В	15.8	44	-	-	-
Westbound Left/Thru/Right	315	В	19.6	164	В	19.6	172	В	19.6	172	-	-	-
Northbound Left/Thru	200	В	13	194	В	17	195	С	22.8	<mark>275</mark>	_	-	_
Northbound Right	50	Α	3.7	m9	Α	6.7	m18	Α	9.4	m29	_	_	_
Southbound Left/Thru	525	В	17.2	99	В	18.6	125	В	19.5	132	_	_	_
Southbound Right	75	В	14.9	11	В	15	16	В	15	16	_	_	_
1) N Quinn Street and Key Boul		<u> </u>			_			_					
Overall Intersection (Signalized	-		_		_					_	С	21	
	500	_	-			-		-	-		В	15.8	45
Eastbound Left/Thru/Right		-	-	-	-	-	-	-	-	-			
Westbound Left/Thru/Right	315	-	-	-	-	-	-	-	-	-	С	23.2	210
Northbound Left/Thru	200	-	-	-	-	-	-	-	-	-	С	24.1	#294
Northbound Right	50	-	-	-	-	-	-	-	-	-	В	10.4	m32
Southbound Left/Thru	525	-	-	-	-	-	-	-	-	-	В	19.7	136
Southbound Right	75	-	-	-	-	-	-	-	-	-	В	15	17
2) N Quinn Street and 18th Stre	et N												
Overall Intersection (Unsignalize		Α	1.2		Α	1.4		Α	3.4		Α	3.4	
Westbound Left/Right Northbound Thru/Right	315 375	B A	11.8 0	8 0	B A	13.4 0	10 0	C A	15.1 0	23 0	B A	15.4 0	33 0
Southbound Left/Thru	200	A	8	0	A	8.3	3	A	8.3	3	A	8.3	3
3) N Quinn Street and Wilson B	oulevard												
Overall Intersection (Signalized)	В	14.6		В	12		В	11.5		В	11.3	
Westbound Thru	325	A B	9.1	76	A	8.1	100	A	7.8	101	A	7.6	105
Westbound Right Southbound Right	85 375	C	18.1 29.3	18 107	B C	11.3 30	12 138	A C	7.8 31.9	11 159	A C	6.6 33.3	m7 168
4) N Pierce Street and Wilson B				-									
Overall Intersection (Signalized)	В	17.8		С	20.9		С	20.3		С	22.2	
Westbound Left/Thru	675	В	13.6	317	В	17.2	428	В	16.8	452	В	19.2	497
Northbound Left	250	D	36.2	161	D	38	210	D	37.4	206	D	37.8	209
5) N Pierce Street and Clarendo Overall Intersection (Signalized		л _В	10 /			17.1			22.7			24	
Eastbound Left/Thru	500	A	18.4 9.1	154	B A	17.1 9.5	183	C B	22.7 12.1	205	C B	24 12.5	217
Eastbound Thru/Right	500	A	9.1	154	Α	9.5	183	В	12.1	205	В	12.5	217
Northbound Thru/Right	185	С	26.6	66	С	27.1	85	С	24.1	83	С	24.1	86
Southbound Left/Thru	250	D	41.1	#271	D	37.3	#280	D	50.1	#371	D	55	m#36
6) New Street and 18 th Street N		ı											
Overall Intersection (Unsignalize	ed)	-	-		-	-		Α	6		Α	5.9	
Eastbound Thru/Right		-	-	-	-	-	-	Α	0	0	Α	0	0
Westbound Left/Thru		-	-	-	-	-	-	Α	7.3	0	Α	7.3	0
Northbound Left/Right		-	-	-	-	-	-	Α	9.7	13	Α	9.7	15
7) New Street and Wilson Boul	evard												
Overall Intersection (Unsignalize	ed)	-	-		-	-		Α	4.2		Α	4.5	
Westbound Thru/Right		-	-	-	-	-	-	Α	0	0	Α	0	0
Southbound Right		-	-	-	-	-	-	D	30.3	140	D	34.3	115
	ii												

			Existing	2016	D	Future V	Vithout ent (2019)	D	Future ' evelopme		Dev	Future velopme	With nt (2025)
Intersection (Movement)	Storage		PM P	eak		PM P			PM Pe		PM Peak		
,,	(ft)	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LOS	Delay	95th Queue
Eastbound Thru/Right	285	Α	0	0	Α	0	0	Α	0	0	Α	0	0
Westbound Left/Thru	150	Α	8.1	5	Α	8.3	5	Α	8.4	5	Α	8.4	8
Northbound Left/Right	200	С	16	35	С	17.4	45	С	19	63	С	20.1	68
9) N Oak Street and 18th Stree		1.						_					
Overall Intersection (Unsignali		A B	3.8	10	A	3.5	4.5	A	4.7	25	А В	4.7	25
Eastbound Left/Right Northbound Left/Thru	850 200	A	10.8 7.7	10 3	B A	11.4 7.8	15 3	B A	12.5 7.8	25 3	A	12.8 7.9	25 3
Southbound Thru/Right	200	Α	0	0	Α	0	0	Α	0	0	Α	0	0
10) N Oak Street and Wilson B	oulevard												
Overall Intersection (Signalized	l)	С	25.9		С	26.8		С	29.7		_	-	
Westbound Thru	125	В	19.1	138	С	20.5	184	С	21.6	201	_	_	_
Westbound Thru/Right	75	В	19.1	138	С	20.5	184	С	21.6	201	_	_	_
Northbound Left/Thru	25	D	50.2	176	D	53.6	188	E	64.5	m#207	_	_	_
Southbound Thru/Right	200	C	29	93	C	30.6	132	C	30.6	132	_	_	_
10) N Oak Street and Wilson B						30.0				102			
Overall Intersection (Signalized	•	Ligat	- -		_	_		С	28.4		С	29	
Westbound Thru	125	_	_	_	_	_	_	С	24	218	С	24.7	234
Westbound Thru/Right	75				_		_	С	24	218	С	24.7	234
	_	-	-	-		-							
Northbound Left/Thru	25	-	-	-	-	-	-	D	52.6	217	D	54	224
Southbound Thru/Right	200	-	-	-	-	-	-	С	22.4	103	С	22.8	108
11) N Oak Street and Clarendo	n Boulevard	1											
Overall Intersection (Signalized	l)	В	14.4		В	15.1		В	17.2		-	-	
Eastbound Left	75	D	47.7	m119	D	48.4	m130	Е	58.7	m#167	-	-	-
Eastbound Thru/Right	550	Α	6.9	m90	Α	8	m122	Α	7.8	m120	-	-	-
Northbound Thru/Right	825	С	28.2	72	С	28	73	С	28	73	-	-	-
Southbound Left/Thru	25	Α	7	9	Α	9.3	15	Α	9.4	15	-	-	-
11) N Oak Street and Clarendo	n Boulevard	(Mit	igated)										
Overall Intersection (Signalized	d)	-	-		-	-		В	14.9		В	15.2	
Eastbound Left	75	_	_	-	_	-	-	D	41	m144	D	41	m148
Eastbound Thru/Right	550	_	_	_	_	_	_	A	9.1	m137	Α	9.7	m149
Northbound Thru/Right	825				_		_	С	23	64	С	23	66
. 0	25		_	_	_	_	_		7.7				16
Southbound Left/Thru 12) N Nash Street and Key Bou		-	-	-	-		-	Α	7.7	15	Α	7.8	10
Overall Intersection (Unsignali		Α	4.4		Α	4.9		Α	6		Α	6.4	
Eastbound Left/Thru	135	Α	8.5	6	Α	8.9	5	Α	9.1	10	Α	9.2	10
Westbound Thru/Right	65	Α	0	0	Α	0	0	Α	0	0	Α	0	0
Southbound Left/Right	650	С	15.7	35	С	21.1	58	D	25.5	73	D	27.4	80
13) N Nash Street (east) and Ko	ey Boulevard	 											
Overall Intersection (Unsignali	zed)	Α	8.8		F	61.3		F	61.3		-	-	
Eastbound Left	65	F	50.4	105	F	414.1	408	F	414.1	408	-	-	-
Eastbound Thru/Right	65	В	14.7	18	С	17	28	С	17	28	-	-	-
	25	Α	0	0	D	29	5	D	29	5	-	-	-
Westbound Left/Thru/Right					l		4.0	_	0.2	10			
Westbound Left/Thru/Right Northbound Left/Thru	400	Α	8.7	10	Α	9.2	18	Α	9.2	18	-	-	-
Northbound Left/Thru	400										_	-	-
Northbound Left/Thru Northbound Thru/Right	400 400	Α	0.3	0	Α	0.6	0	Α	0.6	0	-	-	-
Northbound Left/Thru	400										- - -	- - -	- - -

	Storage		Existing PM P	•	D	Future W evelopme PM P	nt (2019)	De	Future \ evelopme	nt (2019)	Future With Development (2025) PM Peak			
Intersection (Movement)	(ft)	LOS	S Delay	95th Queue	LO	5 Delay	95th Queue	LO	S Delay	95th Queue	LOS	Delay	95th Queue	
Overall Intersection (Signalized)	-	-		-	-		В	13.8		В	13.9		
Eastbound Left	65	-	-	-	-	-	-	D	42.2	166	D	42.3	168	
Eastbound Thru/Right	65	_	-	-	_	_	-	С	33.9	39	С	33.2	39	
Westbound Left/Thru/Right	25	_	_	_	_	_	-	С	27.8	14	С	27.6	14	
Northbound Left/Thru	400	_	_	_	_	_	-	Α	7	136	Α	7.2	142	
Northbound Thru/Right	400	_	-	-	_	_	-	Α	7	136	Α	7.2	142	
Southbound Left/Thru	150	_	-	-	_	_	-	Α	5.8	116	Α	5.9	120	
Southbound Right	150	_	-	-	_	-	-	Α	6.6	31	Α	6.9	32	
14) N Nash Street and Lee High	way													
Overall Intersection (Signalized	1)	В	11.8		В	14.3		В	16.3		В	17.3		
Eastbound Left/Thru	1450	Α	4.8	113	Α	6.2	154	Α	7.3	155	Α	7.6	168	
Eastbound Thru	1450	Α	4.8	113	Α	6.2	154	Α	7.3	155	Α	7.6	168	
Eastbound Thru/Right	1450	Α	4.8	113	Α	6.2	154	Α	7.3	155	Α	7.6	168	
Northbound Thru/Right	650	С	34.1	76	D	40.2	154	D	42.3	205	D	47.1	222	
Southbound Left	125	D	46.6	87	Е	56.4	92	Ε	56	#107	Е	60.9	#114	
Southbound Thru	125	С	33.5	32	С	31.6	38	С	29.5	42	С	29.3	43	
15) N Fort Myer Drive and Lee	Highway NB													
Overall Intersection (Signalized	1)	В	17.9		В	18		В	17.6		В	18.3		
Eastbound Thru	225	С	20.1	126	В	18.2	122	В	17.3	123	В	17.3	126	
Eastbound Right	70	В	17.8	79	С	21.9	125	С	20.7	126	С	23.9	141	
Southbound Left	250	В	18	219	В	18.7	260	В	18.9	265	В	19.8	290	
Southbound Left/Thru	250	В	16	188	В	16.4	216	В	16.5	221	В	16.9	237	

 $[\]mbox{\ensuremath{m}}$ - Volume for 95th percentile queue is metered by upstream signal

Table 11: Saturday Peak Hour Comparison of Capacity Analysis Results

Table 11. Saturday I cak II	Storage (ft)	Existing 2016 Saturday Peak			-	Future W evelopme	/ithout ent (2019)	D	Future \	nt (2019)	Future With Development (2029 Saturday Peak			
Intersection (Movement)		LO	Saturda S Delay	y Peak 95th Queue	LOS	Saturda S Delay	y Peak 95th Queue	LO	Saturday S Delay	95th Queue	L	Saturda .OS elay	y Peak 95th Queue	
1) N Quinn Street and Key Boul	evard													
Overall Intersection (Signalized)		В	13.6		В	13.6		В	13.5		-	-		
Eastbound Left/Thru/Right	500	С	26.3	51	С	26.8	64	С	26.9	65	-	-	-	
Westbound Left/Thru/Right	315	С	28.7	98	С	28.8	105	С	28.8	105	-	-	-	
Northbound Left/Thru	200	Α	5.1	29	Α	5	31	Α	5.5	44	-	-	-	
Northbound Right	50	Α	1.5	m2	Α	1.5	m1	Α	1.8	m2	-	-	-	
Southbound Left/Thru	525	Α	8.3	53	Α	8.5	62	Α	8.5	65	-	-	-	
Southbound Right	75	Α	7.6	11	Α	7.7	14	Α	7.7	14	-	-	-	
1) N Quinn Street and Key Boul	evard (Mitig	ated)											
Overall Intersection (Signalized)	-	-		-	-		-	-	-	В	13.5		
Eastbound Left/Thru/Right	500	-	-	-	-	-	-	-	-	-	С	27	68	
Westbound Left/Thru/Right	315	-	-	-	-	-	-	-	-	-	С	28.4	97	
Northbound Left/Thru	200	-	-	-	-	-	-	-	-	-	Α	5.5	45	
Northbound Right	50	-	-	-	-	-	-	-	-	-	Α	1.8	m2	
Southbound Left/Thru	525	-	-	-	-	-	-	-	-	-	Α	8.6	67	
Southbound Right	75	-	-	-	-	-	-	-	-	-	Α	7.7	14	

^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

			Existing	2016	D	Future W		De	Future \ evelopme		Future With Development (2025)			
Intersection (Movement)	Storage		Saturday	y Peak		Saturda			Saturday		Saturday Peak			
microcolon (movement)	(ft)	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LO	S Delay	95th Queue		LOS	95th Queue	
2) N Quinn Street and 18th Stre	eet N											•		
Overall Intersection (Unsignalis		Α	0.6		Α	0.7		Α	1.8		Α	1.7		
Westbound Left/Right	315	В	10.7	3	В	10.7	3	В	10.8	8	В	10.9	8	
Northbound Thru/Right	375	A	0	0	A	0	0	A	0	0	A	0	0	
Southbound Left/Thru	200	Α	7.7	0	Α	7.8	0	Α	7.8	3	Α	7.9	3	
3) N Quinn Street and Wilson E	oulevard													
Overall Intersection (Signalized	I)	С	21.3		С	21.2		С	20.5		С	20.1		
Westbound Thru	325	В	18.2	100	В	18.5	112	С	20.3	135	С	20.3	141	
Westbound Right	85	С	34.7	35	С	33.7	33	С	27.4	26	С	25.6	25	
Southbound Right	375	В	14.5	128	В	13.8	138	В	13.4	139	В	13.5	142	
4) N Pierce Street and Wilson E														
Overall Intersection (Signalized	l)	В	19.8		В	19.9		В	18.4		В	18.5		
Westbound Left/Thru	675	В	12.3	181	В	13.4	218	В	12.6	220	В	12.8	232	
Northbound Left	250	D	40	170	D	40.2	185	D	38.4	185	D	38.6	190	
5) N Pierce Street and Clarendo	on Boulevard	t												
Overall Intersection (Signalized	l)	Α	8.6		Α	9.1		В	10.1		В	10.4		
Eastbound Left/Thru	500	Α	3.6	99	Α	3.8	115	Α	5.5	146	Α	5.7	157	
Eastbound Thru/Right	500	Α	3.6	99	Α	3.8	115	Α	5.5	146	Α	5.7	157	
Northbound Thru/Right	185	D	36.6	46	D	36.5	53	С	32.6	52	С	32.6	54	
Southbound Left/Thru 6) New Street and 18 th Street N	250	С	30.6	48	С	34.4	56	С	26.8	57	С	28.4	60	
•		ı							. .					
Overall Intersection (Unsignalis	zea) I	-	-		-	-		Α	5.3		Α	5.2		
Eastbound Thru/Right		-	-	-	-	-	-	Α	0	0	Α	0	0	
Westbound Left/Thru		-	-	-	-	-	-	Α	7.3	0	Α	7.3	0	
Northbound Left/Right		-	-	-	-	-	-	Α	9	5	Α	9	5	
7) New Street and Wilson Boul	evard													
Overall Intersection (Unsignalia	zed)	-	-		-	-		Α	1.6		Α	1.5		
Westbound Thru/Right		-	-	-	-	-	-	Α	0	0	Α	0	0	
Southbound Right		-	-	-	-	-	-	В	12.6	18	В	12.9	20	
8) N Oak Street and Key Boulev	/ard													
Overall Intersection (Unsignalia	zed)	Α	4.4		Α	4.6		Α	5.1		Α	5.2		
Eastbound Thru/Right	285	Α	0	0	Α	0	0	Α	0	0	Α	0	0	
Westbound Left/Thru	150	Α	7.8	3	Α	7.8	5	Α	7.8	5	Α	7.8	5	
Northbound Left/Right	200	В	11.2	13	В	11.4	15	В	11.6	18	В	11.7	20	
9) N Oak Street and 18th Stree	t N	1												
Overall Intersection (Unsignalis	zed)	Α	2.2		Α	1.9		Α	2.9		Α	2.9		
Eastbound Left/Right	850	В	10	3	В	10.1	3	В	10.5	8	В	10.6	8	
Northbound Left/Thru	200	Α	7.7	3	Α	7.7	3	Α	7.7	3	Α	7.7	3	
Southbound Thru/Right	200	Α	0	0	Α	0	0	Α	0	0	Α	0	0	
10) N Oak Street and Wilson Bo		۔ ا	22.5						a= c					
Overall Intersection (Signalized	ĺ	С	23.2		С	23.2		С	27.2		-	-		
Westbound Thru	125	В	16	81	В	16.5	100	В	18.6	118	-	-	-	
Westbound Thru/Right	75	В	16	81	В	16.5	100	В	18.6	118	-	-	-	
Northbound Left/Thru	25	D	47.6	130	D	49.3	138	E	57.3	m178	-	-	-	
Southbound Thru/Right	200	С	25.7	51	С	25.6	57	С	26.1	57	-	-	-	
10) N Oak Street and Wilson Bo	10) N Oak Street and Wilson Boulevard (Miti													
Overall Intersection (Signalized	1)	-	-		-	-		С	25		С	25		
Westbound Thru	125	-	-	-	-	-	-	В	17.2	113	В	17.4	120	
Westbound Thru/Right	75	-	-	-	-	-	-	В	17.2	113	В	17.4	120	
Northbound Left/Thru	25	-	-	-	-	-	-	D	53.3	m177	D	53.5	m177	
Southbound Thru/Right	200	_	-	-	_	_	-	С	21.7	53	С	21.9	56	
												_		

Interroption (Mariamant)	Storage		Existing		D	Future Wevelopme Saturda	ent (2019)	D	Future \ evelopmei Saturday	nt (2019)	Future With Development (2025) Saturday Peak		
Intersection (Movement)	(ft)	LO	S Delay	95th Queue	LO	S Delay	95th Queue	LO	S Delay	95th Queue		LOS Jelay	95th Queue
11) N Oak Street and Clarendo	n Boulevard												
Overall Intersection (Signalized	l)	В	15.6		В	15.6		В	17.9		-	-	
Eastbound Left	75	D	51.1	95	D	52.7	105	D	54.9	#165	-	-	-
Eastbound Thru/Right	550	Α	7.3	58	Α	7.4	67	Α	7.1	67	-	-	-
Northbound Thru/Right	825	С	27.3	52	С	27.1	54	С	27.1	54	-	-	-
Southbound Left/Thru	25	В	11.4	11	В	11.9	13	В	11.9	13	-	-	-
11) N Oak Street and Clarendo	n Boulevard	(Miti	igated)										
Overall Intersection (Signalized	(k	-	-		-	-		-	-	-	В	18	
Eastbound Left	75	_	_	-	_	-	-	-	-	-	D	47.5	151
Eastbound Thru/Right	550	_	_	_	_	_	_	_	_	-	Α	7.7	76
Northbound Thru/Right	825	_	_	_	_	_	_	_	_	-	С	26.4	54
Southbound Left/Thru	25	_	_	_	_	_	_	_		_	В	12.8	15
12) N Nash Street and Key Bou	-	-			<u> </u>			-	-	-	ь	12.0	13
Overall Intersection (Unsignali		Α	3.4		Α	3.4		Α	3.9		Α	3.9	
Eastbound Left/Thru	135	Α	7.7	3	Α	7.9	3	Α	8	5	Α	8	5
Westbound Thru/Right	65	Α	0	0	Α	0	0	Α	0	0	Α	0	0
Southbound Left/Right	650	В	10.3	8	В	11.5	15	В	11.9	18	В	12	18
13) N Nash Street (east) and Ko	ey Boulevard	!											
Overall Intersection (Unsignali	zed)	Α	3.6		Α	4.2		Α	4.2		-	-	
Eastbound Left	65	В	12.3	13	С	16.6	28	С	16.6	28	-	-	-
Eastbound Thru/Right	65	В	10.5	5	В	11.2	5	В	11.2	5	-	-	-
Westbound Left/Thru/Right	25	В	11.2	0	В	13.3	0	В	13.3	0	-	-	-
Northbound Left/Thru	400	Α	7.8	3	Α	8	5	Α	8	5	-	-	-
Northbound Thru/Right	400	Α	0.1	0	Α	0.1	0	Α	0.1	0	_	-	-
Southbound Left/Thru	150	Α	7.6	0	Α	7.8	0	Α	7.8	0	_	_	_
Southbound Right	150	Α	0	0	Α	0	0	Α	0	0	_	_	_
13) N Nash Street (east) and Ke	ev Boulevard	l (Mii	igated)										
Overall Intersection (Signalized	-	 	gutcu,		_			В	12.2		В	11.4	
Eastbound Left	ĺ	_	_			_		D	48.2	111	D		112
	65	-	-	-	-	-	-			111		42.3	112
Eastbound Thru/Right	65	-	-	-	-	-	-	D	39.9	32	D	37.9	32
Westbound Left/Thru/Right	25	-	-	-	-	-	-	С	34.4	8	С	33	8
Northbound Left/Thru	400	-	-	-	-	-	-	Α	2.7	35	Α	3.2	36
Northbound Thru/Right	400	-	-	-	-	-	-	Α	2.7	35	Α	3.2	36
Southbound Left/Thru	150	-	-	-	-	-	-	Α	2.8	55	Α	3.2	56
Southbound Right	150	-	-	-	-	-	-	Α	2.7	15	Α	3.1	16
14) N Nash Street and Lee High	•	1											
Overall Intersection (Signalized	i .	Α	9.1		Α	9.2		В	10.2		В	10.5	
Eastbound Left/Thru Eastbound Thru	1450 1450	A	3.3 3.3	79 79	A	3.5 3.5	100 100	A A	4 4	104 104	A	4.1 4.1	115 115
Eastbound Thru/Right	1450	A	3.3 3.3	79 79	A	3.5 3.5	100	A	4	104 104	A	4.1 4.1	115
Northbound Thru/Right	650	D	45.3	51	D	44.2	74	D	43.1	104	D	48	111
Southbound Left	125	D	42.5	69	D	42.2	71	D	40.1	71	D	40.5	72
Southbound Thru 15) N Fort Myor Drive and Lee	125	D	37.1	24	D	37.4	33	D	36.3	42	D	36.3	43
15) N Fort Myer Drive and Lee Overall Intersection (Signalized		В	17.1		В	18.3		В	18.4		В	19.1	
Eastbound Thru	225	A	9	66	A	9.1	66	A	8.8	67	A	8.8	68
Eastbound Right	70	Α	8.2	44	В	10.8	80	В	10.5	81	В	11.6	92
Southbound Left	250	С	24.4	122	С	26.5	170	С	26.9	177	С	28.4	208

Intersection (Movement)		Existing	2016	Future V	Vithout	Future With				Future With		
		LAISTINE	5 2010	Developme	ent (2019)	Development (2019)				Development (202		
	Storage	Saturda	y Peak	Saturda	y Peak		Saturday	Peak		y Peak		
	(ft)	LOS Delay	95th Queue	LOS Delay	95th Queue	LOS	Delay	95th Queue		LOS Jelay	95th Queue	
Southbound Left/Thru	250	C 25.1	161	C 26.6	203	С	26.9	210	С	27.6	229	

 $[\]mbox{\it m}$ - Volume for 95th percentile queue is metered by upstream signal

^{# - 95}th percentile volume exceeds capacity, queue may be longer

 $^{^{\}sim}$ - Volume exceeds capacity, queue is theoretically infinite

TRAFFIC SIGNAL WARRANT ANALYSIS

This section presents the evaluation of the traffic signal control warrant for the intersection of N Nash Street (east) and Key Boulevard. The signal warrant analyses were performed following the procedures outlined in the 2009 Manual on Uniform Traffic Control Devices (MUTCD) for Warrant 3 (peak-hour vehicular volume).

A signal is not needed at the intersection of N Nash Street (east) and Key Boulevard under existing conditions; however, a considerable number of vehicle trips will be added to the study area and this intersection with the 10 planned developments near the West Rosslyn development. Therefore, the future without development (2019) conditions traffic volumes were analyzed. The data points consider the traffic on Key Boulevard as the minor street traffic and the total traffic on N Nash Street as the major street traffic. The major street (N Nash Street) is analyzed as two lanes, and the minor street (Key Boulevard) is analyzed as two lanes. According to the MUTCD, only one warrant needs to be satisfied to allow for the installation of a traffic control signal.

Warrant 3: 4 Hour Vehicular Volume

Typically, hourly traffic volumes of an average day are plotted on the MUTCD Figure 4C-3 and, if any point is above the appropriate curve, the warrant criterion is met. As noted in the MUTCD Figure 4C-3, 150 vehicles per hour apply as the lower threshold volume for a minor-street approach with two lanes.

The future without development traffic conditions are plotted on Figure 28. As shown in **Figure 28**, one of the points lie above the curve for 2 lanes on the major and 2 lanes on the minor streets. The requirement for the warrant to be satisfied is data points for any hour in an average day that lie above the corresponding curve in Figure 4C-3. **Therefore, Warrant 3 is satisfied.**

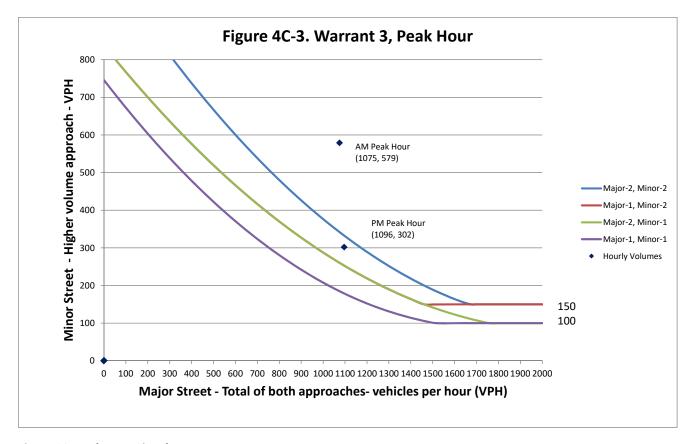


Figure 28: Peak Hour Signal Warrant
TRANSPORTATION MANAGEMENT PLAN

A Transportation Management Plan (TMP) has many components that are tailored to accommodate a given facility with the goal being the reduction of automobile trips by encouraging alternative forms of transportation. A few of the typical TMP components include the establishment of a TMP coordinator, the distribution of transit literature, the establishment of ridesharing programs, and the on-site sale of discounted fare media. Management measures taken by the West Rosslyn project can be monitored and adjusted as needed to continually create opportunities to reduce the amount of vehicular traffic generated by the site.

The TMP will include a schedule and details of implementation and continued operation of the elements in the plan. The location of the site near the Rosslyn Metro Station allows for a TMP that may include, but not be limited to, the following:

Participation and Funding

- a. The Applicant will maintain an active membership in Arlington Transportation Partners (ATP), or successor entity at no cost to the developer, on behalf of the property management company.
- b. The Applicant will designate a member(s) of the building management team as Property Transportation Coordinator (PTC) who will be the primary point of contact with the County and undertake the responsibility for coordinating and completing all TMP obligations.
- c. Provide contributions for Arlington County Commuter Services (ACCS) as negotiated with Arlington County staff during the 4.1 Site Plan process.

Facilities and Improvements

- a. Provide in the new building one information display with transportation related information for residents, employees and visitors, the content/design/location of which shall be mutually agreeable to ACCS and Applicant.
- b. Comply with requirements of the Site Plan conditions to provide bicycle parking/storage facilities and construction worker parking.

Parking Management Plan

- a. Prepare a Parking Management Plan to show how adjacent curb space to the site will be designated for parking and potential on-street loading zones.
- b. Provide effective directional signage to direct residents and visitors to appropriate location on the property, to include provision for the items specified in the Parking Management Plan.

Promotions, Services, Policies

- a. Provide one time, per person, at initial lease-up, to residents and on-site property management employees, the choice of one of the following:
 - i. \$65 Metro fare on a SmarTrip card
 - ii. A one year bikeshare membership
 - iii. A one year carshare membership
- b. Provide website hotlinks to CommuterPage.comTM under a "transportation information" heading from the property manager's websites regarding this development.
- c. Distribute new-resident and new-employee packages, materials provided by Arlington County including site-specific transit-related information to all residents and all on-site employees of the building management company.
- d. Place a reference to the Rosslyn Metro and nearby bus routes in promotional materials and advertisements.
- e. Cooperate, at no cost to the Applicant, with Arlington County to assist the County in implementing a transitadvertising program that will distribute information four times per year to all residents and employees.
- f. Participate in regionally sponsored clean air, transit, and traffic mitigation promotions by posting notice of such promotions in locations within the building.
- g. Provide marketing support to encourage ridesharing.

Performance and Monitoring

- a. Reimburse the County, for a specified amount subject to CPI, for and participate in a transportation performance monitoring study at two years, five years, and at each subsequent five years (at the County's option) after issuance of first Certificate of Occupancy.
- b. During the first year of startup of the TMP and on an annual basis thereafter for a duration of years to be determined, the Applicant will submit an annual letter to the County Manager describing the TDM related activities of the site.

CONCLUSION

The site is located in the West Rosslyn area of Arlington, Virginia, south of 18th Street N, north of Wilson Boulevard, east of N Quinn Street, and west of N Oak Street. The vehicular study area includes three (3) intersections along N Quinn Street, 18th Street N, Key Boulevard, and N Nash Street, four (4) intersections along N Oak Street and Wilson Boulevard, and two (2) intersections along Lee Highway NB and N Pierce Street. Regional access to the site is provided via I-66/US29 and the George Washington Memorial Parkway from the north and east, US 50 (Arlington Blvd) to the south and Wilson Boulevard/Clarendon Boulevard to the west. Immediate vehicular access to the site will be provided via a new north-south street which will bisect the site and connect Wilson Boulevard to 18th Street N.

According to Arlington County's General Land Use Plan (GLUP), this site is currently listed as "High Office-Apartment-Hotel" and "Public Space." The new development will be divided in two parts, with the eastern parcel including one residential tower and space for ground-floor retail while the western parcel will include a tower with office space, additional ground floor retail, and the redevelopment of the existing Rosslyn Highlands Park. The proposed project build-out year is 2019.

The analysis presented in this report supports the following major conclusions:

Existing Conditions (2016)

- The subject site is well-served by transit:
 - The site is approximately less than one-half mile walking distance from the Rosslyn Metro station, which serve the Blue, Orange, and Silver lines.
 - There are four bus stops within one block of the site. These stops are directly served by WMATA (Metrobus)
 and Arlington Transit (ART) routes.
- Vehicular traffic operations in the study area are good overall. All intersection movements within the study area operate at the target Level of Service (LOS) D or better during the AM, PM, and Saturday peak hours, with the exception of the following:
 - N Quinn Street and Wilson Boulevard
 - Southbound right lane (AM peak hour)
 - N Oak Street and Key Boulevard
 - Northbound left/right lane (AM peak hour)
 - N Oak Street and Wilson Boulevard
 - Northbound left/thru lane (AM peak hour)
 - N Nash Street (east) and Key Boulevard (AM peak hour)
 - Eastbound left lane (AM and PM peak hour)
 - N Nash Street and Lee Highway
 - Southbound left lane (AM peak hour)
- None of the movements listed above cause the intersections to operate below acceptable LOS thresholds with the
 exception of the intersection of N Nash Street (east) and Key Boulevard in the AM peak hour. Signalization as
 mitigation at this intersection is considered.

Future Conditions without Development (2019)

- Historical roadway volumes indicate negative growth has occurred in the study area; however a 0.5% annual background growth was applied at the study intersections as per agreement with VDOT and Arlington County. Ten planned background developments in the vicinity of the site were also taken into consideration. The trips generated by these sites were added to the roadway network to reflect future conditions without the proposed development in 2019.
- Under future without development conditions, all movements at the study intersections operate at acceptable levels
 of service consistent with the Existing Conditions scenario, with the exception of the following:
 - N Quinn Street and Key Boulevard
 - Southbound left/thru lane (AM peak hour)
 - N Pierce Street and Clarendon Boulevard
 - Southbound left/thru lane (AM peak hour)
 - N Oak Street and Clarendon Boulevard
 - Eastbound left lane (AM peak hour)
 - N Nash Street (east) and Key Boulevard (PM peak hour)
 - N Nash Street and Lee Highway
 - Southbound left lane (PM peak hour)
- None of the movements listed above cause the intersections to operate below acceptable LOS thresholds with the
 exception of the intersection of N Nash Street (east) and Key Boulevard in the PM peak hour. Signalization as
 mitigation at this intersection is considered.

Future Conditions with Development (2019)

- The proposed mixed-use development will generate approximately 268 net trips in the AM peak hour, 348 net trips in the PM peak hour, and 287 net trips in the Saturday peak hour.
- Under future with development conditions, all intersection movements within the study area operate at acceptable levels of service consistent with the Future without Development scenario, with the exception of the following:
 - N Quinn Street and Key Boulevard (AM peak hour)
 - Southbound left/thru lanes (AM peak hour)
 - N Pierce Street and Clarendon Boulevard
 - Southbound left/thru lane (PM peak hour)
 - N Oak Street and Wilson Boulevard (AM peak hour)
 - Northbound left/thru lane (PM and Saturday peak hour)
 - N Oak Street and Clarendon Boulevard
 - Eastbound left lane (PM peak hour)

All movements caused to operate below acceptable LOS thresholds by the proposed development have been mitigated with signal timing adjustments. No additional mitigations are required.

Future Conditions with Development (2025)

- There are no additional site-added trips between 2019 and 2025.
- Under future (2025) with development conditions, all intersection movements within the study area continue to operate consistent with future (2019) with development results with the exception of the following:
 - N Quinn Street and Key Boulevard
 - Westbound left/thru/right lane (AM peak hour)
 - Southbound left/thru lane (AM peak hour)
- Other movements caused to operate below acceptable LOS thresholds by the proposed development have been mitigated with signal timing adjustments. Although movements at the intersection of N Quinn Street and Key Boulevard exceeds County standards, the 2025 analysis was done as a planning scenario; therefore, no mitigation is proposed at this intersection.

Transportation Management Plan

• A TMP will be required for the project based on the County's requirements. The initial framework for a TMP is included in this report. The ultimate TMP will be determined per the approved site plan conditions.